#### STORMWATER POLLUTION PREVENTION PLAN / STORMWATER MANAGEMENT REPORT FOR 921 SOUNDVIEW DRIVE

Village of Mamaroneck, New York Date: December 29, 2021 (revised) Prepared by: Alan L. Pilch, PE, RLA ALP Engineering & Landscape Architecture, PLLC

#### INTRODUCTION:

The subject property is 22,998.8 square feet in size and is located on the east side of Soundview Drive. The lot is bounded by Soundview Drive on the west, existing residential properties on the north and south, Creek Road (a paper street) and Otter Creek to the east.

The project consists of the following:

- Construction of a new single family residential dwelling in the western portion of the property, with driveway access from Soundview Drive;
- Construction of a pool and pool deck to the east of the house.
- Construction of a stormwater management facility to manage the changes in stormwater runoff from the property.

The limits of disturbance are shown on drawing C-103. The drawing shows that the area of disturbance is 12,700 square feet (0.2916 acres). The proposed land disturbance involves the removal of some of the existing woods and brush areas on the property and the construction of the house, pool, pool deck, as well as the stormwater management facility.

#### STORMWATER MANAGEMENT PLAN

<u>Stormwater Management Plan Requirements</u> - The project stormwater management plan has been designed in accordance with Chapter 294 of the Code of the Village of Mamaroneck and the 2015 New York State *Stormwater Management Design Manual*. Since the land disturbance activity has been calculated to be about 12,700 square feet, as per Chapter 294-8 B.2, the stormwater management plan is to provide stormwater quality and quantity controls (postconstruction stormwater runoff controls). And since the land disturbance area is less than one acre, a SPDES General Permit for Stormwater Runoff from Construction Activity from the NYSDEC will *not* be required.

At present, the property is vacant. With the construction of the house, driveway, pool and pool deck, the total amount of impervious surfaces will be 4,346 s.f. (this number includes the proposed pool deck as impervious to be very conservative). The remainder of the disturbed area will be lawn and landscaped areas.

There are no stormwater management controls on the property at present. Runoff from the entire property is presently conveyed to the east flowing toward Otter Creek. For purposes of analysis, the Design Line is defined as the eastern property line (see Figures 3 and 4).

To manage runoff from the property, it is proposed to install 8 Cultec 330XLHD chambers as four rows of 2 chambers end-to-end. This practice will provide water quantity control (i.e., peak rate attenuation) of the runoff discharged from the property to Otter Crook as well as water quality improvement by capturing and treating the water quality volume.

The runoff conveyed to the chambers will be conveyed first to a diversion structure which will direct the water quality volume to the chambers since the outlet pipe from the diversion structures to the chambers is at a lower elevation that the outlet pipe to the flow spreader. Outflow from the chambers is by infiltration (a conservative 1" per hour rate was used in the modeling) and to two 6" diameter pop-up emitters (see drawing C-102). In the event there is a more intense flow being conveyed to the chambers, the diversion structure will convey a portion of that flow to the flow spreader, and a portion of the flow to the chambers.

The calculations show that the peak rate of runoff to the design line will be less than the existing rate of runoff from the property for all of the modeled storm events.

The text below describes the compliance of the stormwater management plan with Chapter 294 of the Code of the Village of Mamaroneck.

B. Contents of stormwater pollution prevention plans.

(1) All SWPPPs shall provide the following background information and erosion and sediment controls:

(a) Background information about the scope of the project, including location, type and size of project;

The project scope includes:

- Construction of a 2,430 square foot house in the western portion of the property;
- Construction of a 40-foot length paved driveway to the house garage.
- Construction of a stormwater drainage system to consist of a trench drain, catch basins, a stormwater management facility to consist of subsurface chambers, two pop-up emitters and a flow spreader to manage the outflow of runoff.

(b) Site map/construction drawing(s) at a scale not smaller than one inch equals 50 feet, or as otherwise approved by the SMO, for the project, including a general location map. At a minimum, the site map should show the total site area; all improvements; areas of disturbance;

areas that will not be disturbed; existing vegetation; on-site and adjacent off-site surface water(s); wetlands and drainage patterns that could be affected by the construction activity; existing and final slopes; locations of off-site material, waste, borrow or equipment storage areas; and location(s) of the stormwater discharges(s);

The project drawings depict the required elements of the project, as follows: (i) the total site area is depicted on drawing C-101, (ii) all proposed site improvements may be referenced on drawing C-101 and C-102, (iii) areas of disturbance and areas that will not be disturbed may be found on drawing C-103, (iv) general areas of existing vegetation and the location of surveyed trees may be found on drawing C-101, (v) runoff from the project site currently and in the future will be conveyed into Otter Creek which lies to the east of the property; (vii) existing and final slopes may be found on drawing C-102, (viii) the project does not propose to locate any material, waste, or borrow areas off the property; equipment storage areas during construction may be found on drawing C-103.

#### (c) Description of the soil(s) present at the site;

According to the Soils Survey of Putnam and Westchester Counties (Web Soil Survey), the soils over the portion of the property to be impacted by the proposed work consist of Urban land-Charlton-Chatfield complex, rolling, very rocky. The wetland soils consist of Ipswich mucky peat, 0 to 2 percent slopes, very frequently flooded. The Charlton component of the Urban land-Charlton-Chatfield complex soils consist of loam and sandy loam to depth of 60" or so. Chatfield soils consist of loam and flaggy silt loam; the depth to the restrictive layer is 20 to 40 inches. Both soils are in Hydrologic Soils Group B. Group B soils have moderate infiltration rates when thoroughly wetted and consist chiefly of moderately deep to deep, moderately well to well drained soils with moderately fine to moderately coarse textures. These soils have a moderate rate of water transmission.

(d) Construction phasing plan describing the intended sequence of construction activities, including clearing and grubbing, excavation and grading, utility and infrastructure installation and any other activity at the site that results in soil disturbance. Consistent with the New York Standards and Specifications for Erosion and Sediment Control (Erosion Control Manual), not more than five acres shall be disturbed at any one time unless pursuant to an approved SWPPP;

The construction phasing plan may be found on drawing C-103. As noted above, the total area of disturbance is calculated to be 12,700 square feet (0.2916 acres), well below the five acre threshold, and well below 1 acre of disturbance that would trigger the need for a SPDES General Permit.

(e) Description of the pollution prevention measures that will be used to control litter, construction chemicals and construction debris from becoming a pollutant source in stormwater runoff;

The Erosion and Sediment Control Plan incorporates a variety of measures designed to control litter, construction chemicals, and construction debris from becoming a source of pollution. The plan requires the staking of the clearing and grading limit line before the commencement of construction activity. Following the demarcation of the limits of disturbance, a variety of erosion and sediment control measures are to installed in accordance with the plans, including, but not limited to, silt fences and a stabilized construction entrance.

Each contractor and subcontractor who will be involved in soil disturbance and/or stormwater management practice installation shall sign and date a copy of the following certification statement before undertaking any land development activity: "I certify under penalty of law that I understand and agree to comply with the terms and conditions of the stormwater pollution prevention plan. I also understand that it is unlawful for any person to cause or contribute to a violation of water quality standards." The SMO shall provide a form for the contractor/ subcontractor certification statement which shall be signed and returned to the SMO prior to any work taking place.

The certification must include the name and title of the person providing the signature, address and telephone number of the contracting firm; the address (or other identifying description) of the site; and the date the certification is made.

The certification statement(s) shall become part of the SWPPP for the land development activity.

A copy of the SWPPP shall be retained at the site of the land development activity during construction from the date of initiation of construction activities to the date of final stabilization.

As for construction materials, they will be stored in the locations shown on the erosion and sediment control plan, and will be protected by construction fencing as a containment.

Litter control is largely provided by having the maintenance and trash facilities placed inside a fenced-in area. This will reduce the risk of such materials from being washed by rain or blown by wind into the storm drainage system, the public street or toward neighboring properties.

In addition, the construction equipment and material storage area will be located within the portion of the site that is enclosed by the proposed erosion and sediment control measures.

(f) Description of construction and waste materials expected to be stored on-site with updates as appropriate, and a description of controls to reduce pollutants from these materials, including storage practices to minimize exposure of the materials to stormwater, and spill prevention and response;

Construction materials expected to be stored temporarily on site include, but are not limited to, soil stockpiles, stone aggregate for the footings and foundation of the building and pavement, and sod and/or seed to establish lawn for the disturbed areas. These items are not sources of pollution in the short term.

(g) Temporary and permanent structural and vegetative measures to be used for soil stabilization, runoff control and sediment control for each stage of the project from initial land clearing and grubbing to project closeout;

Permanent vegetative measures to be used for soil stabilization may be referenced on the drawings. In the event that site work for the construction is completed at a time of the year that the installation of permanent plantings is not feasible (i.e. late fall, winter and early spring, essentially corresponding to December 1 through April 15), temporary measures are to be installed to prevent erosion, as detailed on drawing C-103 will be implemented.

Temporary Critical Area Plantings, in the event that permanent vegetation cannot be established due to the time of year (i.e. December 1 through April 15), then the seed mixes so noted on drawing C-103 are to be used to stabilize the ground surface until such time as permanent stabilization can be achieved.

(h) A site map/construction drawing(s) specifying the location(s), size(s) and length(s) of each erosion and sediment control practice;

Drawing C-103 depicts the location, size and length of each erosion and sediment control measure to be implemented during construction.

(*i*) Dimensions, material specifications and installation details for all erosion and sediment control practices, including the sitting and sizing of any temporary sediment basins;

Dimensions, material specifications and installation details for all erosion and sediment control practices may be referenced on drawing C-103. Temporary sediment basins are not proposed for the site construction.

(j) Temporary practices that will be converted to permanent control measures;

There are no temporary practices that will be converted to permanent control measures.

(k) Implementation schedule for staging temporary erosion and sediment control practices, including the timing of initial placement and duration that each practice should remain in place;

The erosion control narrative on drawing C-103 provides the implementation schedule for the staging temporary erosion and sediment control practices, describes the time when practices will

be placed and the duration that each practice is to remain in place. The work described in the narrative will occur in a single phase.

(*l*) Maintenance schedule to ensure continuous and effective operation of the erosion and sediment control practice;

The erosion control narrative on drawing C-103 provides the maintenance schedule for each erosion and sediment control practice.

(m) Name(s) of the receiving water(s);

All runoff from the subject property is conveyed to Otter Creek.

(n) Delineation of SWPPP implementation responsibilities for each part of the site;

Implementation of the SWPPP erosion control measures will be the responsibility of the property owner.

(o) Description of structural practices designed to divert flows from exposed soils, store flows, or otherwise limit runoff and the discharge of pollutants from exposed areas of the site to the degree attainable; and

There are no structural practices being proposed to divert runoff flow around the construction area.

(p) Any existing data that describes the stormwater runoff at the site.

This stormwater management plan has been developed for the property that quantifies the existing and future condition site runoff. No other existing data is available which quantifies the flows from the property.

(2) Post-construction runoff controls for new development and redevelopment projects.

As noted above, all runoff from the subject property is conveyed directly into Otter Creek. The project will result in an increase of about 4,346 s.f., in the amount of impervious surfaces following the work (for purposes of this calculation, the deck is considered to be impervious).

Runoff from the impervious surfaces (house roof and driveway) will be collected via roof drain leaders and in a trench drain and directed into the chambers via underground storm drainage pipes. Two 6-inch diameter pop-up emitter from the chambers will attenuate the flows discharged from the chambers.

(a) All information in § 294-7 of this chapter;

See above for the description of the requirements of § 294-7.

(b) Description of each post-construction stormwater management practice (practices shall be as approved in Chapter 4 of the New York State DEC Stormwater Design Manual);

The post-construction stormwater management practice consists of the construction of subsurface detention system. The practice is to consist of eight (8) Cultec 330XLHD chambers, arranged as four rows of two chambers end-to-end. Cultec chambers are constructed of high density polyethylene. Each chamber is 8.5 feet in length by 52 inches in width and 30.5 inches in height. When installed, the stormwater management practice will be 17.5 feet in length by 20.83 feet in width.

(c) Site map/construction drawing(s) showing the specific location(s) and size(s) of each postconstruction stormwater management practice;

The location of the subsurface infiltration chambers may be referenced on drawing C-102.

(d) Hydrologic and hydraulic analysis for all structural components of the stormwater management system for the applicable design storms;

The storm pipe table on sheet C-102 shows that for each segment of pipe to be installed the capacity will exceed the anticipated flows for the 25-year storm event.

(e) Comparison of post-development stormwater runoff conditions with pre-development conditions;

Appendix A provides the calculations which compare the pre-development conditions and the post-development conditions.

	Existing Condition	Future Condition
1-year storm	0.08	0.07
2-year storm	0.22	0.19
10-year storm	0.81	0.63
25-year storm	1.38	1.06
100-year storm	2.68	2.03

#### Table 1. Flows to Design Point

As can be seen in the calculations, the peak rates of runoff from the property to the Design Point will decrease over all of the modeled storm event. This will have the beneficial impact on runoff flows to the Beaver Swamp Brook.

The water quality volume calculation for the drainage area that conveys runoff to the stormwater management practice is calculated to be 425 cubic feet. The proposed 8 Cultec 330XLHD chambers will contain 794 cubic feet of runoff. Infiltration of runoff conveyed to the chambers will also occur. With an estimated very conservative soil percolation rate of 1.00 inches per hour, the volume of percolation is calculated to be 0.175 cubic feet per square foot per day. The bed area of the installed chambers will be 364.58 square feet. Therefore, the volume of percolation is calculated to be (364.58 square feet x 0.175 cubic feet per square foot per day) 63.8 cubic feet. Together, the chambers will capture and treat (794 c.f. + 63.8 c.f.) 857 cubic feet, in excess of the 425 cubic feet calculated for the water quality volume. The HydroCAD modeling also shows that there is no outflow from the chambers (except for exfiltration) during the 1-year storm event. This indicates that the runoff from the impervious surfaces will be fully treated by infiltration into the site's soils.

(f) Dimensions, material specifications and installation details for each post-construction stormwater management practice;

The proposed stormwater management practice to consist of subsurface chambers will be 17.5' in length x 20.83' in width, including the crushed stone installed around the perimeter of the chambers.

(g) Maintenance schedule to ensure continuous and effective operation of each post-construction stormwater management practice;

The maintenance schedule may be referenced on the drawings.

(h) Maintenance easements, if applicable, to ensure access to all stormwater management practices at the site for the purpose of inspection and repair. Easements shall be recorded on the plan and shall remain in effect with transfer of title to the property;

Maintenance easements are not required or proposed.

(i) Inspection and maintenance agreement binding on all subsequent landowners served by the on-site stormwater management practices in accordance with § 294-9 of this chapter;

Inspection and maintenance agreements would be made part of the approvals of the project.

(*j*) The SWPPP shall be prepared by a New York State licensed professional engineer, certified professional in erosion and sediment control (CPESC), or licensed landscape architect and must be signed by the professional preparing the plan, who shall certify that the design of all stormwater management practices meets the requirements in this chapter. [Amended 9-22-2014 by L.L. No. 17-2014, effective 10-30-2014]

The SWPPP has been prepared by a professional engineer and landscape architect and certifies that the design of all stormwater management practices meets the requirements.

FIGURES





UIC—Urban land-Charlton-Chatfield complex, rolling, very rocky lp—lpswich mucky peat, 0 to 2 percent slopes, very frequently flooded

> Figure 2 **SOILS MAP** Scale: Not to Scale





SUPPORTING DOCUMENTATION

#### Table 1 921 Soundview Drive Water Quality Volume Calculation

	Area
FDA-2A TO SW PRACTICE	<u>(in sq feet)</u>
Impervious surfaces	2,379
<u>Lawn, good, HSG B</u>	<u>3,101</u>
TOTAL	5,480

	Area
FDA-2B TO SW PRACTICE	<u>(in sq feet)</u>
Impervious surfaces	<u>1,037</u>
TOTAL	1,037

#### WQv Calculation

1.5 inches
6,517 s.f.
3,416 s.f.
52.4 %
0.52
0.010 ac-feet
425.0 cu feet

## **Extreme Precipitation Tables**

### Northeast Regional Climate Center

Data represents point estimates calculated from partial duration series. All precipitation amounts are displayed in inches.

Smoothing	No
State	New York
Location	
Longitude	73.720 degrees West
Latitude	40.950 degrees North
Elevation	0 feet
Date/Time	Thu, 19 Aug 2021 12:12:57 -0400

### **Extreme Precipitation Estimates**

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.34	0.52	0.63	0.85	1.05	1.26	1yr	0.90	1.23	1.48	1.92	2.38	2.86	3.22	1yr	2.53	3.09	3.58	4.31	4.94	1yr
2yr	0.40	0.62	0.77	1.04	1.28	1.53	2yr	1.11	1.50	1.75	2.26	2.81	3.45	3.87	2yr	3.05	3.72	4.27	5.08	5.75	2yr
5yr	0.47	0.73	0.91	1.25	1.59	1.89	5yr	1.37	1.85	2.16	2.80	3.49	4.32	4.89	5yr	3.82	4.70	5.45	6.39	7.13	5yr
10yr	0.54	0.83	1.03	1.44	1.86	2.22	10yr	1.61	2.17	2.54	3.29	4.12	5.12	5.84	10yr	4.53	5.62	6.56	7.60	8.39	10yr
25yr	0.65	0.99	1.23	1.76	2.31	2.75	25yr	2.00	2.68	3.14	4.08	5.13	6.41	7.40	25yr	5.68	7.11	8.37	9.57	10.40	25yr
50yr	0.74	1.13	1.41	2.03	2.73	3.23	50yr	2.35	3.15	3.70	4.81	6.06	7.61	8.84	50yr	6.73	8.50	10.08	11.39	12.25	50yr
100yr	0.86	1.30	1.62	2.35	3.22	3.80	100yr	2.78	3.71	4.35	5.69	7.17	9.03	10.58	100yr	7.99	10.17	12.14	13.57	14.42	100yr
200yr	0.99	1.49	1.88	2.72	3.80	4.47	200yr	3.28	4.37	5.11	6.72	8.48	10.73	12.65	200yr	9.49	12.17	14.63	16.16	16.99	200yr
500yr	1.20	1.78	2.29	3.33	4.74	5.56	500yr	4.09	5.43	6.35	8.40	10.59	13.48	16.04	500yr	11.93	15.43	18.73	20.38	21.10	500yr

### **Lower Confidence Limits**

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.27	0.41	0.50	0.68	0.83	0.89	1yr	0.72	0.87	1.32	1.56	1.94	2.57	2.88	1yr	2.28	2.77	3.25	3.87	4.53	1yr
2yr	0.39	0.61	0.75	1.01	1.25	1.50	2yr	1.08	1.47	1.71	2.19	2.75	3.35	3.76	2yr	2.97	3.61	4.14	4.92	5.60	2yr
5yr	0.44	0.68	0.84	1.15	1.46	1.77	5yr	1.26	1.73	2.01	2.60	3.26	4.02	4.54	5yr	3.56	4.37	5.05	5.93	6.68	5yr
10yr	0.48	0.74	0.92	1.29	1.66	2.00	10yr	1.43	1.96	2.27	2.96	3.70	4.62	5.22	10yr	4.09	5.02	5.87	6.75	7.61	10yr
25yr	0.54	0.82	1.01	1.45	1.91	2.36	25yr	1.65	2.30	2.67	3.47	4.38	5.53	6.28	25yr	4.89	6.03	7.18	8.01	9.05	25yr
50yr	0.58	0.88	1.09	1.57	2.12	2.66	50yr	1.83	2.60	3.03	3.93	4.97	6.34	7.22	50yr	5.61	6.94	8.38	9.06	10.32	50yr
100yr	0.63	0.95	1.19	1.72	2.36	2.99	100yr	2.03	2.92	3.44	4.46	5.66	7.26	8.29	100yr	6.43	7.97	9.77	10.27	11.77	100yr
200yr	0.69	1.03	1.31	1.90	2.64	3.38	200yr	2.28	3.30	3.92	5.06	6.46	8.34	9.54	200yr	7.38	9.17	11.42	11.63	13.45	200yr
500yr	0.77	1.15	1.48	2.15	3.05	3.99	500yr	2.63	3.90	4.66	6.05	8.25	10.00	11.49	500yr	8.85	11.05	14.05	13.67	16.04	500yr

### **Upper Confidence Limits**

	5min	10min	15min	30min	60min	120min		1hr	2hr	3hr	6hr	12hr	24hr	48hr		1day	2day	4day	7day	10day	
1yr	0.37	0.57	0.70	0.94	1.16	1.38	1yr	1.00	1.35	1.61	2.13	2.62	3.13	3.51	1yr	2.77	3.38	3.84	4.65	5.27	1yr
2yr	0.43	0.66	0.81	1.10	1.35	1.60	2yr	1.17	1.57	1.83	2.37	2.94	3.56	4.02	2yr	3.15	3.87	4.41	5.30	5.95	2yr
5yr	0.51	0.79	0.98	1.35	1.72	2.02	5yr	1.48	1.97	2.34	3.00	3.74	4.62	5.26	5yr	4.09	5.06	5.86	6.85	7.61	5yr
10yr	0.61	0.94	1.16	1.62	2.09	2.42	10yr	1.81	2.37	2.85	3.63	4.53	5.64	6.47	10yr	4.99	6.22	7.27	8.42	9.20	10yr
25yr	0.77	1.17	1.46	2.08	2.74	3.11	25yr	2.36	3.04	3.71	4.69	5.83	7.33	8.50	25yr	6.49	8.17	9.67	11.08	11.82	25yr
50yr	0.91	1.39	1.73	2.49	3.35	3.75	50yr	2.89	3.67	4.53	5.69	7.06	8.96	10.45	50yr	7.93	10.05	12.00	13.65	14.28	50yr
100yr	1.10	1.66	2.07	3.00	4.11	4.54	100yr	3.55	4.44	5.51	6.92	8.54	10.93	12.87	100yr	9.68	12.37	14.88	16.80	17.27	100yr
200yr	1.31	1.97	2.50	3.62	5.04	5.49	200yr	4.35	5.36	6.73	8.41	10.35	13.35	15.85	200yr	11.81	15.24	18.46	20.71	20.87	200yr
500yr	1.68	2.50	3.21	4.67	6.64	7.04	500yr	5.73	6.88	8.75	10.89	13.27	17.39	20.88	500yr	15.39	20.08	24.53	27.34	26.81	500yr





#### **CULTEC Stormwater Design Calculator**

Date: December 29, 2021			
Project Info	ormation:		Calculations
'Arcangelo Property 21 Soundview Drive amaroneck (V) ew York			Alan Pilch, PE, RLA ALP Engineering & Land Arch P.O. Box 845 Ridgefield CT 06877
		RECHARGER 330XLHD	(475) 215-5343
			alan@eaec-inc.com
Recharger : Chamber Spe	330XLHD cifications	2	alan@eaec-inc.com Breakdown of Sto Recharger 330XLHD
Recharger : Chamber Spe Height Width Length	330XLHD scifications 30.5 inches 52.0 inches 8.50 feet		Breakdown of Sto Recharger 330XLHD Within Chambers Within Feed Connectors Within Store
Recharger : Chamber Spe Height Width Length Installed Length	330XLHD scifications 30.5 inches 52.0 inches 8.50 feet 7.00 feet		Breakdown of Sto Recharger 330XLHD Within Chambers Within Feed Connectors Within Store Total Storage Provideo
Recharger : Chamber Spe Height Width Length Installed Length Bare Chamber Volume	330XLHD           scifications           30.5         inches           52.0         inches           8.50         feet           7.00         feet           52.21         cu. feet		Breakdown of Sto Recharger 330XLHD Within Chambers Within Feed Connectors Within Stomage Provided Total Storage Required Total Storage Required

#### **Materials List**

Recharger 330XLHD										
Total Number of Chambers Required	8	pieces								
Starter Chambers	4	pieces								
Intermediate Chambers	0	pieces								
End Chambers	4	pieces								
HVLV FC-24 Feed Connectors	6	pieces								
CULTEC No. 410 Non-Woven Geotextile	139	sq. yards								
CULTEC No. 4800 Woven Geotextile	42	feet								
Stone	31	cu. yards								

Based on 2 Internal Manifolds





Bed Layout Information										
Number of Rows Wide	4	pieces								
Number of Chambers Long	2	pieces								
Chamber Row Width	18.83	feet								
Chamber Row Length	15.50	feet								
Bed Width	20.83	feet								
Bed Length	17.50	feet								
Bed Area Required	364.58	sq. feet								
Length of Separator Row	N/A	feet								

Bed detail for reference only. Not project specific. Not to scale.



Conceptual graphic only. Not job specific.

	Cross Section Table Reference		
Α	Depth of Stone Base	6.0	inches
в	Chamber Height	30.5	inches
с	Depth of Stone Above Units	6.0	inches
D	Depth of 95% Compacted Fill	10.0	inches
E	Max. Depth Allowed Above the Chamber	12.00	feet
F	Chamber Width	52.0	inches
G	Center to Center Spacing	4.83	feet
н	Effective Depth	3.54	feet
I	Bed Depth	4.38	feet



#### **CULTEC Stage-Storage Calculations**

Project Number: 0

Date: December 30, 2021

Project Information:
D'Arcangelo Property
921 Soundview Drive
Mamaroneck (V)
New York 10543

Chamber Model -	Recharger 330XLHD	)
Number of Rows-	4	units
Total Number of Chambers -	8	units
HVLV FC-24 Feed Connectors-	6	units
Stone Void -	40	%
Stone Base -	6	inches
Stone Above Units -	6	inches
Area -	364.58	ft2
Base of Stone Elevation -	13.71	

Recharger 330XLHD Incremental Storage Volumes														
Height	Height of System Chamber Volume		r Volume	HVLV Feed Connector Volume		Stone V	olume	Cumulativ Volu	e Storage me	Total Cumu Storage Vo	lative lume	Eleva	tion	
in	mm	ft <sup>3</sup>	m <sup>3</sup>	ft3	m3	ft <sup>3</sup>	m <sup>3</sup>	ft <sup>3</sup>	m <sup>3</sup>	ft <sup>3</sup>	m <sup>3</sup>		m	
41.5 40.5 39.5 38.5 37.5 36.5 36.0 35.0 34.0	1054 1029 1003 978 953 927 914 889 864 828	0.0 0.0 0.0 0.0 1.2 3.2 5.2 7 7	0.0 0.0 0.0 0.0 0.0 0.0 0.1 0.1	0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	12.2 12.2 12.2 12.2 6.1 11.7 10.9 10.1	0.3 0.3 0.3 0.3 0.2 0.3 0.3 0.3 0.3 0.3	12.153 12.153 12.153 12.153 12.153 6.080 12.860 14.050 15.278	0.3 0.3 0.3 0.3 0.2 0.4 0.4 0.4	/84.28 772.12 759.97 747.82 735.66 723.51 717.43 704.57 690.52	22.21 21.86 21.52 21.18 20.83 20.49 20.32 19.95 19.55	17.170 17.090 17.000 16.920 16.840 16.750 16.710 16.630 16.540	14.76 14.74 14.71 14.69 14.66 14.64 14.62 14.60 14.57	Top of Chamber Elevation
33.0 32.0 31.0 29.0 28.0 27.0 26.0 25.0	838 813 787 762 737 711 686 660 635	9.3 10.7 11.8 12.8 13.7 14.4 15.1 15.7	0.2 0.3 0.3 0.4 0.4 0.4 0.4 0.4 0.4	0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	9.1 8.4 7.9 7.4 6.7 6.4 6.1 5.9	0.3 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.2	16.765 17.733 18.588 19.258 19.853 20.374 20.820 21.230 21.602	0.5 0.5 0.5 0.6 0.6 0.6 0.6 0.6	673.24 658.48 640.75 622.16 602.90 583.05 562.67 541.85 520.62	19.12 18.65 18.14 17.62 17.07 16.51 15.93 15.34 14.74	16.460 16.380 16.290 16.210 16.130 16.040 15.960 15.880 15.790	14.55 14.52 14.50 14.47 14.45 14.42 14.40 14.37 14.35	
24.0 23.0 22.0 21.0 20.0 19.0 18.0 17.0	610 584 559 533 508 483 457 432	16.4 16.8 17.5 18.2 18.4 18.5 18.7 18.8	0.5 0.5 0.5 0.5 0.5 0.5 0.5 0.5	0.0 0.0 0.0 0.0 0.0 0.0 0.3 0.3	0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	5.6 5.4 5.1 4.9 4.8 4.7 4.7 4.7	0.2 0.2 0.1 0.1 0.1 0.1 0.1 0.1	21.974 22.234 22.680 23.090 23.164 23.276 23.680 23.694	0.6 0.6 0.7 0.7 0.7 0.7 0.7 0.7	499.02 477.05 454.81 432.13 409.04 385.88 362.60 338.92	14.13 13.51 12.88 12.24 11.58 10.93 10.27 9.60	15.710 15.630 15.540 15.380 15.290 15.210 15.130	14.32 14.29 14.27 14.24 14.22 14.19 14.17 14.14	
16.0 15.0 14.0 13.0 12.0 11.0 10.0 9.0	406 381 356 330 305 279 254 229 203	18.8 19.0 19.4 19.9 20.0 20.0 20.1 20.2	0.5 0.5 0.6 0.6 0.6 0.6 0.6 0.6	0.3 0.2 0.2 0.2 0.2 0.2 0.2 0.2 0.1	0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	4.6 4.6 4.4 4.2 4.2 4.1 4.1 4.1	0.1 0.1 0.1 0.1 0.1 0.1 0.1 0.1	23.720 23.791 24.045 24.331 24.353 24.381 24.392 24.381	0.7 0.7 0.7 0.7 0.7 0.7 0.7 0.7	315.23 291.51 267.72 243.67 219.34 194.99 170.61 146.22	8.93 8.25 7.58 6.90 6.21 5.52 4.83 4.14	15.040 14.960 14.880 14.790 14.710 14.630 14.540 14.460 14.460	14.12 14.09 14.07 14.04 14.01 13.99 13.96 13.94	
8.0 7.0 6.0 5.0 4.0 3.0 2.0 1.0	203 178 152 127 102 76 51 25	20.3 20.6 0.0 0.0 0.0 0.0 0.0 0.0	0.6 0.6 0.0 0.0 0.0 0.0 0.0 0.0	0.1 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0	4.0 3.9 12.2 12.2 12.2 12.2 12.2 12.2 12.2	0.1 0.1 0.3 0.3 0.3 0.3 0.3 0.3 0.3	24.377 24.542 12.153 12.153 12.153 12.153 12.153 12.153 12.153	0.7 0.7 0.3 0.3 0.3 0.3 0.3 0.3 0.3	121.84 97.46 72.92 60.76 48.61 36.46 24.31 12.15	3.45 2.76 2.06 1.72 1.38 1.03 0.69 0.34	14.380 14.290 14.210 14.130 14.040 13.960 13.880 13.790	13.91 13.89 13.86 13.84 13.81 13.79 13.76 13.74	Bottom of Chamber Elev
0.0 -1.0 -2.0 -3.0 -4.0 -5.0	0	0.0	0.0	0.0	0.0	0.0	0.0	0.000	0.0	0.00	0.00	13.710	13.71	Bottom of Stone Elevatio

CULTEC, Inc. P.O. Box 280 Brookfield, CT 06804

Recharger 330XLHD Incremental Storage Volumes													
Height of System Chamber Volume			HVLV Feed Connecto	r Volume	Stone V	/olume	Cumulativ Volu	e Storage Ime	Total Cumu Storage Vo	lative olume	Eleva	tion	
<b>in</b> -6.0	mm	ft <sup>3</sup>	m <sup>3</sup>	ft3	m3	ft <sup>3</sup>	m <sup>3</sup>	ft <sup>3</sup>	m <sup>3</sup>	ft <sup>3</sup>	m <sup>3</sup>	ft	m
-58.0 -59.0 0.0	0	0.0	0.0	0.0	0.0	0.0	0.0	0.000	0.0	0.00	0.00	0.000	13.71
0.0 0.0 0.0	0 0 0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.000 0.000 0.000	0.0 0.0 0.0	0.00 0.00 0.00	0.00 0.00 0.00	0.000 0.000 0.000	13.71 13.71 13.71
0.0 0.0 0.0	0 0 0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.000 0.000 0.000	0.0 0.0 0.0	0.00 0.00 0.00	0.00 0.00 0.00	0.000 0.000 0.000	13.71 13.71 13.71
0.0 0.0 0.0	0 0 0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.000 0.000 0.000	0.0 0.0 0.0	0.00 0.00 0.00	0.00 0.00 0.00	0.000 0.000 0.000	13.71 13.71 13.71
0.0 0.0 0.0	0 0 0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.000 0.000 0.000	0.0 0.0 0.0	0.00 0.00 0.00	0.00 0.00 0.00	0.000 0.000 0.000	13.71 13.71 13.71
0.0 0.0 0.0	0 0 0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.000 0.000 0.000	0.0 0.0 0.0	0.00 0.00 0.00	0.00 0.00 0.00	0.000 0.000 0.000	13.71 13.71 13.71
0.0 0.0 0.0	0 0 0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.000 0.000 0.000	0.0 0.0 0.0	0.00 0.00 0.00	0.00 0.00 0.00	0.000 0.000 0.000	13.71 13.71 13.71
0.0 0.0 0.0	0 0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.000 0.000 0.000	0.0 0.0 0.0	0.00 0.00 0.00	0.00 0.00 0.00	0.000 0.000 0.000	13.71 13.71 13.71

### Westchester County, New York

# Ip—Ipswich mucky peat, 0 to 2 percent slopes, very frequently flooded

#### Map Unit Setting

National map unit symbol: 2tyqj Elevation: 0 to 10 feet Mean annual precipitation: 36 to 71 inches Mean annual air temperature: 39 to 55 degrees F Frost-free period: 140 to 250 days Farmland classification: Not prime farmland

#### Map Unit Composition

Ipswich and similar soils: 90 percent Minor components: 10 percent Estimates are based on observations, descriptions, and transects of the mapunit.

#### **Description of Ipswich**

#### Setting

Landform: Tidal marshes Landform position (three-dimensional): Dip Down-slope shape: Linear Across-slope shape: Linear Parent material: Partially- decomposed herbaceous organic material

#### **Typical profile**

Oe - 0 to 42 inches: mucky peat Oa - 42 to 59 inches: muck

#### **Properties and qualities**

Slope: 0 to 2 percent
Depth to restrictive feature: More than 80 inches
Drainage class: Very poorly drained
Runoff class: Negligible
Capacity of the most limiting layer to transmit water
(Ksat): Moderately low to very high (0.14 to 99.90 in/hr)
Depth to water table: About 0 inches
Frequency of flooding: Very frequent
Frequency of ponding: None
Calcium carbonate, maximum content: 5 percent
Maximum salinity: Nonsaline to strongly saline (0.7 to 111.6 mmhos/cm)
Sodium adsorption ratio, maximum: 20.0
Available water supply, 0 to 60 inches: Very high (about 26.6 inches)

#### Interpretive groups

Land capability classification (irrigated): None specified

USDA

Land capability classification (nonirrigated): 8w Hydrologic Soil Group: A/D Ecological site: R144AY001CT - Tidal Salt Low Marsh mesic very frequently flooded, R144AY002CT - Tidal Salt High Marsh mesic very frequently flooded Hydric soil rating: Yes

#### **Minor Components**

#### Pawcatuck

Percent of map unit: 5 percent Landform: Tidal marshes Landform position (three-dimensional): Dip Down-slope shape: Linear Across-slope shape: Linear Ecological site: R144AY001CT - Tidal Salt Low Marsh mesic very frequently flooded, R144AY002CT - Tidal Salt High Marsh mesic very frequently flooded Hydric soil rating: Yes

#### Westbrook

Percent of map unit: 5 percent Landform: Tidal marshes Landform position (three-dimensional): Dip Down-slope shape: Linear Across-slope shape: Linear Ecological site: R144AY001CT - Tidal Salt Low Marsh mesic very frequently flooded, R144AY002CT - Tidal Salt High Marsh mesic very frequently flooded Hydric soil rating: Yes

### **Data Source Information**

Soil Survey Area: Westchester County, New York Survey Area Data: Version 16, Jun 11, 2020

### Westchester County, New York

# UIC—Urban land-Charlton-Chatfield complex, rolling, very rocky

#### Map Unit Setting

National map unit symbol: bd7n Elevation: 0 to 1,000 feet Mean annual precipitation: 46 to 50 inches Mean annual air temperature: 46 to 52 degrees F Frost-free period: 115 to 215 days Farmland classification: Not prime farmland

#### Map Unit Composition

Urban land: 40 percent Charlton and similar soils: 20 percent Chatfield and similar soils: 15 percent Minor components: 25 percent Estimates are based on observations, descriptions, and transects of the mapunit.

#### **Description of Charlton**

#### Setting

Landform: Hills, ridges, till plains Landform position (two-dimensional): Shoulder Landform position (three-dimensional): Crest Down-slope shape: Convex Across-slope shape: Convex Parent material: Acid loamy till derived mainly from schist, gneiss, or granite

#### Typical profile

H1 - 0 to 8 inches: loam

H2 - 8 to 24 inches: sandy loam

H3 - 24 to 60 inches: sandy loam

#### Properties and qualities

Slope: 2 to 15 percent Depth to restrictive feature: More than 80 inches Drainage class: Well drained Capacity of the most limiting layer to transmit water (Ksat): Moderately high to high (0.57 to 5.95 in/hr) Depth to water table: More than 80 inches Frequency of flooding: None Frequency of ponding: None Available water supply, 0 to 60 inches: Moderate (about 7.5 inches)

#### Interpretive groups

Land capability classification (irrigated): None specified Hydrologic Soil Group: B

*Ecological site:* F144AY034CT - Well Drained Till Uplands *Hydric soil rating:* No

#### **Description of Chatfield**

#### Setting

Landform: Hills, ridges Landform position (two-dimensional): Shoulder Landform position (three-dimensional): Crest Down-slope shape: Convex Across-slope shape: Convex Parent material: Loamy till derived mainly from granite, gneiss, or schist

#### **Typical profile**

H1 - 0 to 7 inches: loam
H2 - 7 to 24 inches: flaggy silt loam
H3 - 24 to 28 inches: unweathered bedrock

#### **Properties and qualities**

Slope: 2 to 15 percent
Depth to restrictive feature: 20 to 40 inches to lithic bedrock
Drainage class: Well drained
Capacity of the most limiting layer to transmit water (Ksat): Low to high (0.01 to 5.95 in/hr)
Depth to water table: More than 80 inches
Frequency of flooding: None
Frequency of ponding: None
Calcium carbonate, maximum content: 1 percent
Available water supply, 0 to 60 inches: Low (about 3.2 inches)

#### Interpretive groups

Land capability classification (irrigated): None specified Hydrologic Soil Group: B Ecological site: F144AY034CT - Well Drained Till Uplands Hydric soil rating: No

#### **Minor Components**

#### Sutton

Percent of map unit: 5 percent Hydric soil rating: No

#### Leicester

Percent of map unit: 5 percent Landform: Depressions Hydric soil rating: No

#### **Rock outcrop**

Percent of map unit: 5 percent Hydric soil rating: Unranked

#### Udorthents

Percent of map unit: 5 percent Hydric soil rating: No

USDA

#### Hollis

Percent of map unit: 2 percent Hydric soil rating: No

#### Sun

Percent of map unit: 2 percent Landform: Depressions Hydric soil rating: Yes

#### Palms

Percent of map unit: 1 percent Landform: Swamps, marshes Hydric soil rating: Yes

### **Data Source Information**

Soil Survey Area: Westchester County, New York Survey Area Data: Version 16, Jun 11, 2020

Appendix A

Stormwater Management Report Hydrographs and Routings



### 921 Soundview SW Mgmt\_12-28-2021.2

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Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (inches)	AMC
1	1-year	Type III 24-hr		Default	24.00	1	2.86	2
2	2-year	Type III 24-hr		Default	24.00	1	3.45	2
3	10-year	Type III 24-hr		Default	24.00	1	5.12	2
4	25-year	Type III 24-hr		Default	24.00	1	6.41	2
5	100-year	Type III 24-hr		Default	24.00	1	9.03	2

### **Rainfall Events Listing**

\_\_\_\_\_

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### Area Listing (all nodes)

Are	ea CN	Description
(acre	s)	(subcatchment-numbers)
0.19	91 61	>75% Grass cover, Good, HSG B (2S, 3S)
0.44	15 56	Brush, Fair, HSG B (1S)
0.13	38 48	Brush, Good, HSG B (2S)
0.23	31 73	Brush, Good, HSG D (1S, 2S)
0.02	29 61	Deck (use lawn for under deck), HSG B (2S)
0.05	55 98	Impervious surfaces, HSG B (3S)
0.00	98 98	Pool, HSG B (2S)
0.02	24 98	Roofs, HSG B (6S)
1.12	20 63	TOTAL AREA

### 921 Soundview SW Mgmt\_12-28-2021.2

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HSG-A	HSG-B	HSG-C	HSG-D	Other	Total	Ground	Subcatchment
(acres)	(acres)	(acres)	(acres)	(acres)	(acres)	Cover	Numbers
 0.000	0.191	0.000	0.000	0.000	0.191	>75% Grass cover, Good	2S
							, 20
0.000	0.445	0.000	0.000	0.000	0.445	Brush, Fair	33 1S
0.000	0.138	0.000	0.231	0.000	0.368	Brush, Good	1S
							, 2S
0.000	0.029	0.000	0.000	0.000	0.029	Deck (use lawn for under deck)	2S
0.000	0.055	0.000	0.000	0.000	0.055	Impervious surfaces	3S
0.000	0.009	0.000	0.000	0.000	0.009	Pool	2S
0.000	0.024	0.000	0.000	0.000	0.024	Roofs	6S
0.000	0.890	0.000	0.231	0.000	1.120	TOTAL AREA	

### Ground Covers (all nodes)

921 Soundview SW Mgmt 12-28-2021.2

Type III 24-hr 1-year Rainfall=2.86"

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Time span=0.00-48.00 hrs, dt=0.02 hrs, 2401 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: XDA-1 TO DESIGN LINE Runoff Area=24,402 sf 0.00% Impervious Runoff Depth=0.28" Flow Length=260' Tc=6.0 min CN=60 Runoff=0.08 cfs 0.013 af

Subcatchment 2S: FDA-1 TO DESIGN LINE Runoff Area=17,884 sf 2.19% Impervious Runoff Depth=0.31" Flow Length=399' Tc=6.0 min CN=61 Runoff=0.07 cfs 0.011 af

Subcatchment 3S: FDA-2A TO	SW Runoff Area=5,480 sf 43.41% Impervious Runoff Depth=0.98"
	Tc=6.0 min CN=77 Runoff=0.14 cfs 0.010 af
Subcatchment 6S: FDA-2B	Runoff Area=1,037 sf 100.00% Impervious Runoff Depth=2.63" Tc=6.0 min CN=98 Runoff=0.07 cfs 0.005 af
Reach 7R: Reach-1	Avg. Flow Depth=0.00' Max Vel=0.00 fps Inflow=0.00 cfs 0.000 af n=0.400 L=55.0' S=0.1273 '/' Capacity=0.43 cfs Outflow=0.00 cfs 0.000 af

 Reach 8R: Reach-2
 Avg. Flow Depth=0.00'
 Max Vel=0.00 fps
 Inflow=0.00 cfs
 0.000 af

 n=0.400
 L=64.0'
 S=0.0391 '/'
 Capacity=0.24 cfs
 Outflow=0.00 cfs
 0.000 af

Pond 4P: SW MGMT PRACTICE Peak Elev=15.02' Storage=337 cf Inflow=0.20 cfs 0.015 af Discarded=0.01 cfs 0.015 af Primary=0.00 cfs 0.000 af Outflow=0.01 cfs 0.015 af

Pond 6P: Diversion Structure 2A Peak Elev=17.56' Inflow=0.14 cfs 0.010 af Primary=0.14 cfs 0.010 af Secondary=0.00 cfs 0.000 af Outflow=0.14 cfs 0.010 af

 Pond 7P: Diversion Structure 2B
 Peak Elev=17.92'
 Inflow=0.07 cfs
 0.005 af

 Primary=0.07 cfs
 0.005 af
 Secondary=0.00 cfs
 0.000 af
 Outflow=0.07 cfs
 0.005 af

Link 5L: DESIGN LINE

Inflow=0.07 cfs 0.011 af Primary=0.07 cfs 0.011 af

Total Runoff Area = 1.120 ac Runoff Volume = 0.039 af Average Runoff Depth = 0.42" 92.20% Pervious = 1.033 ac 7.80% Impervious = 0.087 ac

#### 921 Soundview SW Mgmt\_12-28-2021.2

Type III 24-hr 1-year Rainfall=2.86"

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#### Summary for Subcatchment 1S: XDA-1 TO DESIGN LINE

Runoff = 0.08 cfs @ 12.16 hrs, Volume= 0.013 af, Depth= 0.28"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs Type III 24-hr 1-year Rainfall=2.86"

A	rea (sf)	CN E	Description			
	5,028	73 E	Brush, Goo	d, HSG D		
	19,374	56 E	Brush, Fair,	HSG B		
	24,402	60 V	Veighted A	verage		
	24,402	1	00.00% Pe	ervious Are	a	
Тс	Length	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
3.7	31	0.0516	0.14		Sheet Flow, A-B	
					Grass: Dense n= 0.240 P2= 3.45"	
0.2	36	0.2000	3.13		Shallow Concentrated Flow, B-C	
					Short Grass Pasture Kv= 7.0 fps	
0.2	44	0.1954	3.09		Shallow Concentrated Flow, C-D	
					Short Grass Pasture Kv= 7.0 fps	
0.6	70	0.0786	1.96		Shallow Concentrated Flow, D-E	
					Short Grass Pasture Kv= 7.0 fps	
0.2	30	0.2166	3.26		Shallow Concentrated Flow, E-F	
					Short Grass Pasture Kv= 7.0 fps	
0.2	49	0.0667	3.87		Shallow Concentrated Flow, F-G	
					Grassed Waterway Kv= 15.0 fps	
0.9					Direct Entry, Tc Factor	
6.0	260	Total				

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#### Subcatchment 1S: XDA-1 TO DESIGN LINE

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Type III 24-hr 1-year Rainfall=2.86"

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#### Summary for Subcatchment 2S: FDA-1 TO DESIGN LINE

Runoff = 0.07 cfs @ 12.14 hrs, Volume= 0. Routed to Link 5L : DESIGN LINE

0.011 af, Depth= 0.31"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs Type III 24-hr 1-year Rainfall=2.86"

	A	rea (sf)	CN I	Description							
		5,028	73	Brush, Goo	d, HSG D						
		5,993	48	Brush, Goo	d, HSG B						
		5,229	61	>75% Gras	s cover, Go	ood, HSG B					
*		1,242	61	Deck (use l	awn for und	der deck), HSG B					
*		392	98	Pool, HSG B							
		17,884	61	Weighted A	verage						
		17,492	9	97.81% Pe	rvious Area						
		392	1	2.19% Impe	ervious Area	а					
	Тс	Length	Slope	Velocity	Capacity	Description					
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	3.7	35	0.0657	0.16		Sheet Flow, A-B					
						Grass: Dense n= 0.240 P2= 3.45"					
	1.0	220	0.0659	3.85		Shallow Concentrated Flow, B-C					
						Grassed Waterway Kv= 15.0 fps					
	0.3	65	0.0769	4.16		Shallow Concentrated Flow, C-D					
						Grassed Waterway Kv= 15.0 fps					
	0.2	30	0.2166	3.26		Shallow Concentrated Flow, E-F					
						Short Grass Pasture Kv= 7.0 fps					
	0.2	49	0.0667	3.87		Shallow Concentrated Flow, F-G					
						Grassed Waterway Kv= 15.0 fps					
_	0.6					Direct Entry, Tc Factor					
	6.0	399	Total								

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#### Subcatchment 2S: FDA-1 TO DESIGN LINE

921 Soundview SW Mgmt\_12-28-2021.2

Type III 24-hr 1-year Rainfall=2.86"

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#### Summary for Subcatchment 3S: FDA-2A TO SW PRACTICE

Runoff = 0.14 cfs @ 12.10 hrs, Volume= Routed to Pond 6P : Diversion Structure 2A 0.010 af, Depth= 0.98"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs Type III 24-hr 1-year Rainfall=2.86"

	Area (sf)	CN	Description			
*	2,379	98	Impervious surfaces, HSG B			
	3,101	61	>75% Grass cover, Good, HSG B			
	5,480	77	Weighted Average			
	3,101		56.59% Pervious Area			
	2,379		43.41% Imp	pervious Are	rea	
۲ miı)	rc Length n) (feet)	Slop (ft/f	e Velocity t) (ft/sec)	Capacity (cfs)	Description	
6	.0		· · · ·		Direct Entry,	

#### Subcatchment 3S: FDA-2A TO SW PRACTICE


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#### Summary for Subcatchment 6S: FDA-2B

Runoff = 0.07 cfs @ 12.08 hrs, Volume= Routed to Pond 7P : Diversion Structure 2B 0.005 af, Depth= 2.63"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs Type III 24-hr 1-year Rainfall=2.86"



Type III 24-hr 1-year Rainfall=2.86"

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### Summary for Reach 7R: Reach-1

Inflow Area = 0.150 ac, 52.42% Impervious, Inflow Depth = 0.00" for 1-year event Inflow 0.00 cfs @ 0.00 hrs, Volume= 0.000 af = Outflow 0.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min = 0.00 cfs @ Routed to Reach 8R : Reach-2 Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min Peak Storage= 0 cf @ 0.00 hrs Average Depth at Peak Storage= 0.00' Bank-Full Depth= 0.10' Flow Area= 1.5 sf, Capacity= 0.43 cfs 15.00' x 0.10' deep channel, n= 0.400 Sheet flow: Woods+light brush Side Slope Z-value= 3.0 '/' Top Width= 15.60' Length= 55.0' Slope= 0.1273 '/' Inlet Invert= 15.00', Outlet Invert= 8.00'



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Reach 7R: Reach-1

Type III 24-hr 1-year Rainfall=2.86"

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### Summary for Reach 8R: Reach-2

Inflow Area = 0.150 ac, 52.42% Impervious, Inflow Depth = 0.00" for 1-year event Inflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af Outflow = 0.00 cfs @ 0.00 hrs, Volume= 0.000 af, Atten= 0%, Lag= 0.0 min Routed to Link 5L : DESIGN LINE

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs Max. Velocity= 0.00 fps, Min. Travel Time= 0.0 min Avg. Velocity = 0.00 fps, Avg. Travel Time= 0.0 min

Peak Storage= 0 cf @ 0.00 hrs Average Depth at Peak Storage= 0.00' Bank-Full Depth= 0.10' Flow Area= 1.5 sf, Capacity= 0.24 cfs

15.00' x 0.10' deep channel, n= 0.400 Sheet flow: Woods+light brush Side Slope Z-value= 3.0 '/' Top Width= 15.60' Length= 64.0' Slope= 0.0391 '/' Inlet Invert= 8.00', Outlet Invert= 5.50'



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## Reach 8R: Reach-2

Type III 24-hr 1-year Rainfall=2.86"

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## Summary for Pond 4P: SW MGMT PRACTICE

Inflow Area	=	0.15	0 ac, 5	2.42% Imp	ervious,	Inflow D	Depth =	1.24"	for 1	l-yea	ar event	
Inflow :	=	0.20	cfs @	12.09 hrs,	Volume	=	0.015	af		-		
Outflow :	=	0.01	cfs @	12.16 hrs,	Volume	=	0.015	af, At	ten= 95	5%,	Lag= 4.1	min
Discarded :	=	0.01	cfs @	12.16 hrs,	Volume	=	0.015	af				
Primary :	=	0.00	cfs @	0.00 hrs,	Volume	=	0.000	af				
Routed t	o Reacl	n 7R :	Reach	ı <b>-</b> 1								

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs Peak Elev= 15.02' @ 15.48 hrs Surf.Area= 421 sf Storage= 337 cf

Plug-Flow detention time= 349.6 min calculated for 0.015 af (100% of inflow) Center-of-Mass det. time= 349.6 min (1,174.7 - 825.1)

Volume	Invert	Avail.Storage	Storage Description
#1B	13.71'	332 cf	20.83'W x 17.50'L x 3.54'H Field A
			1,291 cf Overall - 462 cf Embedded = 829 cf x 40.0% Voids
#2B	14.21'	462 cf	Cultec R-330XLHD x 8 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 4 rows
#3	14.50'	156 cf	6.00'D x 2.75'H Vertical Cone/Cylinder x 2
		949 cf	Total Available Storage

Storage Group B created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	17.00'	6.0" Horiz. Orifice/Grate X 2.00 C= 0.600
#2	Discarded	13.71'	Limited to weir flow at low heads 1.000 in/hr Exfiltration over Horizontal area

**Discarded OutFlow** Max=0.01 cfs @ 12.16 hrs HW=14.51' (Free Discharge) **2=Exfiltration** (Exfiltration Controls 0.01 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=13.71' (Free Discharge) ☐ 1=Orifice/Grate (Controls 0.00 cfs)

Type III 24-hr 1-year Rainfall=2.86"

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## Pond 4P: SW MGMT PRACTICE - Chamber Wizard Field A

#### Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 4 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

2 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 15.50' Row Length +12.0" End Stone x 2 = 17.50' Base Length 4 Rows x 52.0" Wide + 6.0" Spacing x 3 + 12.0" Side Stone x 2 = 20.83' Base Width 6.0" Stone Base + 30.5" Chamber Height + 6.0" Stone Cover = 3.54' Field Height

8 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 4 Rows = 462.0 cf Chamber Storage

1,291.2 cf Field - 462.0 cf Chambers = 829.3 cf Stone x 40.0% Voids = 331.7 cf Stone Storage

Chamber Storage + Stone Storage = 793.7 cf = 0.018 af Overall Storage Efficiency = 61.5% Overall System Size = 17.50' x 20.83' x 3.54'

8 Chambers 47.8 cy Field 30.7 cy Stone





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### Pond 4P: SW MGMT PRACTICE





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### Summary for Pond 6P: Diversion Structure 2A

[57] Hint: Peaked at 17.56' (Flood elevation advised)

Inflow Area	a =	0.126 ac, 4	3.41% Impe	ervious,	Inflow Dept	:h= (	0.98"	for	1-ye	ar event	
Inflow	=	0.14 cfs @	12.10 hrs,	Volume=	= 0.	010 a	f		•		
Outflow	=	0.14 cfs @	12.10 hrs,	Volume=	= 0.	010 a	lf, At	ten= 0	%,	Lag= 0.0	) min
Primary	=	0.14 cfs @	12.10 hrs,	Volume=	= 0.	010 a	f			•	
Routed	to Pond	4P : SW MG	MT PRACT	ICE							
Secondary	=	0.00 cfs @	0.00 hrs,	Volume=	= 0.	000 a	f				
Routed	to Reac	h 7R : Reach	ı-1								

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs / 2 Peak Elev= 17.56' @ 12.10 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	17.33'	<b>8.0" Round Culvert to SW Mgmt Facility</b> L= 3.5' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 17.33' / 15.92' S= 0.4029 '/' Cc= 0.900 n= 0.012, Flow Area= 0.35 sf
#2	Secondary	17.58'	<b>8.0" Round Culvert to Flow Spreader</b> L= 30.5' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 17.58' / 15.08' S= 0.0820 '/' Cc= 0.900 n= 0.012, Flow Area= 0.35 sf

Primary OutFlow Max=0.14 cfs @ 12.10 hrs HW=17.56' (Free Discharge) ←1=Culvert to SW Mgmt Facility (Inlet Controls 0.14 cfs @ 1.29 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=17.33' (Free Discharge) 2=Culvert to Flow Spreader (Controls 0.00 cfs)

Hydrograph Inflow 0.14 cfs Outflow Primary 0.14 cfs Inflow Area=0.126 ac Secondary 0.14 cfs 0.15 Peak Elev=17.56' 0.14 0.13 0.12 0.11 0.1 0.09 (cfs) 0.08 Flow 0.07 0.06 0.05 0.04 0.03 0.02 0.01 0 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 ż 4 Ó Time (hours)

## Pond 6P: Diversion Structure 2A

## Pond 6P: Diversion Structure 2A



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## Summary for Pond 7P: Diversion Structure 2B

[57] Hint: Peaked at 17.92' (Flood elevation advised)

Inflow Area	a =	0.024 ac,10	0.00% Impe	ervious, I	nflow Depth	= 2.6	53" for	1-year ev	vent
Inflow	=	0.07 cfs @	12.08 hrs,	Volume=	0.0	)5 af		-	
Outflow	=	0.07 cfs @	12.08 hrs,	Volume=	0.0	)5 af,	Atten= 0	%, Lag=	0.0 min
Primary	=	0.07 cfs 🥘	12.08 hrs,	Volume=	0.0	)5 af		Ū	
Routed	to Pond	4P : SW MG	MT PRACT	ICE					
Secondary	=	0.00 cfs @	0.00 hrs,	Volume=	0.0	00 af			
Routed	to Reac	h 7R : Reach	-1						

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs Peak Elev= 17.92' @ 12.08 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	17.75'	6.0" Round Culvert to SW Mgmt Facility L= 3.5' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 17.75' / 16.25' S= 0.4286 '/' Cc= 0.900 n= 0.012, Flow Area= 0.20 sf
#2	Secondary	18.00'	6.0" Round Culvert to Flow Spreader L= 27.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 18.00' / 15.25' S= 0.1019 '/' Cc= 0.900 n= 0.012, Flow Area= 0.20 sf

Primary OutFlow Max=0.07 cfs @ 12.08 hrs HW=17.92' (Free Discharge) ←1=Culvert to SW Mgmt Facility (Inlet Controls 0.07 cfs @ 1.11 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=17.75' (Free Discharge) 2=Culvert to Flow Spreader (Controls 0.00 cfs)



### Pond 7P: Diversion Structure 2B

## Pond 7P: Diversion Structure 2B



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Type III 24-hr 1-year Rainfall=2.86"

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## Summary for Link 5L: DESIGN LINE

Inflow Area	a =	0.560 ac,	15.61% Impe	ervious,	Inflow Depth	= 0.2	23" for 1-y	ear event
Inflow	=	0.07 cfs @	12.14 hrs,	Volume	= 0.0	11 af		
Primary	=	0.07 cfs @	12.14 hrs,	Volume	= 0.0	11 af,	Atten= 0%,	Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs



### Link 5L: DESIGN LINE

Type III 24-hr 2-year Rainfall=3.45"

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Time span=0.00-48.00 hrs, dt=0.02 hrs, 2401 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: XDA-1 TO DESIGN LINE Runoff Area=24,402 sf 0.00% Impervious Runoff Depth=0.51" Flow Length=260' Tc=6.0 min CN=60 Runoff=0.22 cfs 0.024 af

Subcatchment 2S: FDA-1 TO DESIGN LINE Runoff Area=17,884 sf 2.19% Impervious Runoff Depth=0.55" Flow Length=399' Tc=6.0 min CN=61 Runoff=0.19 cfs 0.019 af

Subcatchment 3S: FDA-2A TO SW	Runoff Area=5,480 sf 43.41% Impervious Runoff Depth=1.39"
	I c=6.0 min CN=77 Runoff=0.20 cfs 0.015 af
Subcatchment 6S: FDA-2B	Runoff Area=1,037 sf 100.00% Impervious Runoff Depth=3.22"
	Tc=6.0 min CN=98 Runoff=0.08 cfs 0.006 af
Reach 7R: Reach-1	Avg. Flow Depth=0.00' Max Vel=0.01 fps Inflow=0.00 cfs 0.000 af
n=0.40	00 L=55.0' S=0.1273 '/' Capacity=0.43 cfs Outflow=0.00 cfs 0.000 af

 Reach 8R: Reach-2
 Avg. Flow Depth=0.00'
 Max Vel=0.01 fps
 Inflow=0.00 cfs
 0.000 af

 n=0.400
 L=64.0'
 S=0.0391 '/'
 Capacity=0.24 cfs
 Outflow=0.00 cfs
 0.000 af

Pond 4P: SW MGMT PRACTICE Peak Elev=15.57' Storage=519 cf Inflow=0.28 cfs 0.021 af Discarded=0.01 cfs 0.021 af Primary=0.00 cfs 0.000 af Outflow=0.01 cfs 0.021 af

Pond 6P: Diversion Structure 2A Peak Elev=17.61' Inflow=0.20 cfs 0.015 af Primary=0.20 cfs 0.015 af Secondary=0.00 cfs 0.000 af Outflow=0.20 cfs 0.015 af

 Pond 7P: Diversion Structure 2B
 Peak Elev=17.94'
 Inflow=0.08 cfs
 0.006 af

 Primary=0.08 cfs
 0.006 af
 Secondary=0.00 cfs
 0.000 af
 Outflow=0.08 cfs
 0.006 af

Link 5L: DESIGN LINE

Inflow=0.19 cfs 0.019 af Primary=0.19 cfs 0.019 af

Total Runoff Area = 1.120 acRunoff Volume = 0.064 afAverage Runoff Depth = 0.68"92.20% Pervious = 1.033 ac7.80% Impervious = 0.087 ac

Type III 24-hr 2-year Rainfall=3.45"

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## Summary for Subcatchment 1S: XDA-1 TO DESIGN LINE

Runoff = 0.22 cfs @ 12.12 hrs, Volume= 0.024 af, Depth= 0.51"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs Type III 24-hr 2-year Rainfall=3.45"

A	rea (sf)	CN E	Description			
	5,028	73 E	Brush, Goo	d, HSG D		
	19,374	56 E	Brush, Fair,	HSG B		_
	24,402	60 V	Veighted A	verage		
	24,402	1	00.00% Pe	ervious Are	a	
Тс	Length	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		_
3.7	31	0.0516	0.14		Sheet Flow, A-B	
					Grass: Dense n= 0.240 P2= 3.45"	
0.2	36	0.2000	3.13		Shallow Concentrated Flow, B-C	
					Short Grass Pasture Kv= 7.0 fps	
0.2	44	0.1954	3.09		Shallow Concentrated Flow, C-D	
					Short Grass Pasture Kv= 7.0 fps	
0.6	70	0.0786	1.96		Shallow Concentrated Flow, D-E	
	0.0	0.0400	0.00		Short Grass Pasture Kv= 7.0 fps	
0.2	30	0.2166	3.26		Shallow Concentrated Flow, E-F	
0.0	40	0.0007	0.07		Short Grass Pasture KV= 7.0 fps	
0.2	49	0.0667	3.87		Shallow Concentrated Flow, F-G	
0.0					Grassed Waterway KV= 15.0 fps	
0.9					Direct Entry, IC Factor	_
6.0	260	Total				

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#### Subcatchment 1S: XDA-1 TO DESIGN LINE

Type III 24-hr 2-year Rainfall=3.45"

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# Summary for Subcatchment 2S: FDA-1 TO DESIGN LINE

Runoff	=	0.19 cfs @	12.12 hrs,	Volume=	
Route	d to L	ink 5L : DESIGN	LINE		

0.019 af, Depth= 0.55"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs Type III 24-hr 2-year Rainfall=3.45"

_	A	rea (sf)	CN I	Description							
		5,028	73 I	Brush, Goo	d, HSG D						
		5,993	48 I	48 Brush, Good, HSG B							
		5,229 61 >75% Grass cover, Good, HSG B									
*		1,242	61 I	Deck (use l	awn for und	der deck), HSG B					
*		392	98 I	Pool, HSG	В						
		17,884	61 \	Weighted A	verage						
		17,492	ę	97.81% Pervious Area							
		392		2.19% Impe	ervious Area	а					
	_				<b>•</b> •	-					
	IC	Length	Slope	Velocity	Capacity	Description					
	(min)	(feet)	(ft/ft)	(ft/sec)	(cts)						
	3.7	35	0.0657	0.16		Sheet Flow, A-B					
						Grass: Dense n= 0.240 P2= 3.45"					
	1.0	220	0.0659	3.85		Shallow Concentrated Flow, B-C					
						Grassed Waterway Kv= 15.0 fps					
	0.3	65	0.0769	4.16		Shallow Concentrated Flow, C-D					
						Grassed Waterway Kv= 15.0 fps					
	0.2	30	0.2166	3.26		Shallow Concentrated Flow, E-F					
		10	0 0007	0.07		Short Grass Pasture Kv= 7.0 fps					
	0.2	49	0.0667	3.87		Shallow Concentrated Flow, F-G					
	0.0					Grassed Waterway KV= 15.0 fps					
_	0.6					Direct Entry, IC Factor					
	6.0	399	Total								

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#### Subcatchment 2S: FDA-1 TO DESIGN LINE

Type III 24-hr 2-year Rainfall=3.45"

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### Summary for Subcatchment 3S: FDA-2A TO SW PRACTICE

Runoff = 0.20 cfs @ 12.09 hrs, Volume= Routed to Pond 6P : Diversion Structure 2A 0.015 af, Depth= 1.39"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs Type III 24-hr 2-year Rainfall=3.45"

	Area (sf)	CN	Description							
*	2,379	98	Impervious	surfaces, H	HSG B					
	3,101	61	>75% Gras	75% Grass cover, Good, HSG B						
	5,480	77	Weighted A	verage						
	3,101		56.59% Per	vious Area	3					
	2,379		43.41% Imp	pervious Are	rea					
mi]	Гс Length n) (feet)	Slop (ft/f	e Velocity t) (ft/sec)	Capacity (cfs)	Description					
6	.0				Direct Entry,					

### Subcatchment 3S: FDA-2A TO SW PRACTICE



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Type III 24-hr 2-year Rainfall=3.45"

#### Summary for Subcatchment 6S: FDA-2B

Runoff = 0.08 cfs @ 12.08 hrs, Volume= Routed to Pond 7P : Diversion Structure 2B 0.006 af, Depth= 3.22"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs Type III 24-hr 2-year Rainfall=3.45"



Type III 24-hr 2-year Rainfall=3.45"

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### Summary for Reach 7R: Reach-1

Inflow Area = 0.150 ac, 52.42% Impervious, Inflow Depth = 0.00" for 2-year event Inflow 0.00 cfs @ 12.09 hrs, Volume= 0.000 af = 0.00 cfs @ 13.31 hrs, Volume= Outflow 0.000 af, Atten= 95%, Lag= 73.0 min = Routed to Reach 8R : Reach-2 Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs Max. Velocity= 0.01 fps, Min. Travel Time= 69.2 min Avg. Velocity = 0.01 fps, Avg. Travel Time= 69.2 min Peak Storage= 1 cf @ 12.16 hrs Average Depth at Peak Storage= 0.00', Surface Width= 15.00' Bank-Full Depth= 0.10' Flow Area= 1.5 sf, Capacity= 0.43 cfs 15.00' x 0.10' deep channel, n= 0.400 Sheet flow: Woods+light brush Side Slope Z-value= 3.0 '/' Top Width= 15.60' Length= 55.0' Slope= 0.1273 '/' Inlet Invert= 15.00', Outlet Invert= 8.00'



Reach 7R: Reach-1

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Reach 7R: Reach-1

Type III 24-hr 2-year Rainfall=3.45"

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#### Summary for Reach 8R: Reach-2

Inflow Area = 0.150 ac, 52.42% Impervious, Inflow Depth = 0.00" for 2-year event Inflow = 0.00 cfs @ 13.31 hrs, Volume= 0.000 af Outflow = 0.00 cfs @ 17.30 hrs, Volume= 0.000 af, Atten= 74%, Lag= 239.6 min Routed to Link 5L : DESIGN LINE

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs Max. Velocity= 0.01 fps, Min. Travel Time= 145.3 min Avg. Velocity = 0.01 fps, Avg. Travel Time= 145.3 min

Peak Storage= 0 cf @ 14.88 hrs Average Depth at Peak Storage= 0.00', Surface Width= 15.00' Bank-Full Depth= 0.10' Flow Area= 1.5 sf, Capacity= 0.24 cfs

15.00' x 0.10' deep channel, n= 0.400 Sheet flow: Woods+light brush Side Slope Z-value= 3.0 '/' Top Width= 15.60' Length= 64.0' Slope= 0.0391 '/' Inlet Invert= 8.00', Outlet Invert= 5.50'



### Reach 8R: Reach-2

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Reach 8R: Reach-2

Type III 24-hr 2-year Rainfall=3.45"

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## Summary for Pond 4P: SW MGMT PRACTICE

Inflow Area =	0.150 ac,	52.42% Impervious,	nflow Depth = 1.6	38" for 2-year event
Inflow =	0.28 cfs @	12.09 hrs, Volume=	: 0.021 af	
Outflow =	0.01 cfs @	12.08 hrs, Volume=	: 0.021 af,	Atten= 96%, Lag= 0.0 min
Discarded =	0.01 cfs @	12.08 hrs, Volume=	: 0.021 af	
Primary =	0.00 cfs @	0.00 hrs, Volume=	: 0.000 af	
Routed to	Reach 7R : Reac	h-1		
Routing by St	or-Ind method, Ti	me Span= 0.00-48.00	hrs, dt= 0.02 hrs	
Peak Elev= 1	5.57' @ 16.28 hrs	Surf.Area= 421 sf	Storage= 519 cf	

Plug-Flow detention time= 528.2 min calculated for 0.021 af (100% of inflow)

Center-of-Mass det. time= 528.2 min (1,348.1 - 819.9)

Volume	Invert	Avail.Storage	Storage Description
#1B	13.71'	332 cf	20.83'W x 17.50'L x 3.54'H Field A
			1,291 cf Overall - 462 cf Embedded = 829 cf x 40.0% Voids
#2B	14.21'	462 cf	Cultec R-330XLHD x 8 Inside #1
			Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf
			Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap
			Row Length Adjustment= +1.50' x 7.45 sf x 4 rows
#3	14.50'	156 cf	6.00'D x 2.75'H Vertical Cone/Cylinder x 2
		949 cf	Total Available Storage

Storage Group B created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	17.00'	6.0" Horiz. Orifice/Grate X 2.00 C= 0.600
			Limited to weir flow at low heads
#2	Discarded	13.71'	1.000 in/hr Exfiltration over Horizontal area

**Discarded OutFlow** Max=0.01 cfs @ 12.08 hrs HW=14.54' (Free Discharge) **2=Exfiltration** (Exfiltration Controls 0.01 cfs)

Primary OutFlow Max=0.00 cfs @ 0.00 hrs HW=13.71' (Free Discharge) ☐ 1=Orifice/Grate (Controls 0.00 cfs)

Type III 24-hr 2-year Rainfall=3.45"

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## Pond 4P: SW MGMT PRACTICE - Chamber Wizard Field A

#### Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 4 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

2 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 15.50' Row Length +12.0" End Stone x 2 = 17.50' Base Length 4 Rows x 52.0" Wide + 6.0" Spacing x 3 + 12.0" Side Stone x 2 = 20.83' Base Width 6.0" Stone Base + 30.5" Chamber Height + 6.0" Stone Cover = 3.54' Field Height

8 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 4 Rows = 462.0 cf Chamber Storage

1,291.2 cf Field - 462.0 cf Chambers = 829.3 cf Stone x 40.0% Voids = 331.7 cf Stone Storage

Chamber Storage + Stone Storage = 793.7 cf = 0.018 af Overall Storage Efficiency = 61.5% Overall System Size = 17.50' x 20.83' x 3.54'

8 Chambers 47.8 cy Field 30.7 cy Stone





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### Pond 4P: SW MGMT PRACTICE





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### Summary for Pond 6P: Diversion Structure 2A

[57] Hint: Peaked at 17.61' (Flood elevation advised)

Inflow Area	a =	0.126 ac, 4	3.41% Impe	ervious,	Inflow De	pth =	1.39	" for	2-yea	ar event	
Inflow	=	0.20 cfs @	12.09 hrs,	Volume=	=	0.015	af		•		
Outflow	=	0.20 cfs @	12.09 hrs,	Volume=	=	0.015	af, A	Atten= 0	%, L	.ag= 0.0	min
Primary	=	0.20 cfs @	12.09 hrs,	Volume=	=	0.015	af			•	
Routed	to Pond	4P : SW MG	<b>MT PRACT</b>	ICE							
Secondary	=	0.00 cfs @	12.09 hrs,	Volume=	=	0.000	af				
Routed	to Reac	h 7R : Reach	n-1								

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs / 2 Peak Elev= 17.61' @ 12.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	17.33'	<b>8.0" Round Culvert to SW Mgmt Facility</b> L= 3.5' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 17.33' / 15.92' S= 0.4029 '/' Cc= 0.900 n= 0.012, Flow Area= 0.35 sf
#2	Secondary	17.58'	8.0" Round Culvert to Flow Spreader L= 30.5' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 17.58' / 15.08' S= 0.0820 '/' Cc= 0.900 n= 0.012, Flow Area= 0.35 sf

Primary OutFlow Max=0.20 cfs @ 12.09 hrs HW=17.61' (Free Discharge) ←1=Culvert to SW Mgmt Facility (Inlet Controls 0.20 cfs @ 1.42 fps)

Secondary OutFlow Max=0.00 cfs @ 12.09 hrs HW=17.61' (Free Discharge) 2=Culvert to Flow Spreader (Inlet Controls 0.00 cfs @ 0.46 fps)

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### Pond 6P: Diversion Structure 2A





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### Summary for Pond 7P: Diversion Structure 2B

[57] Hint: Peaked at 17.94' (Flood elevation advised)

Inflow Area	a =	0.024 ac,10	0.00% Imp	ervious,	Inflow De	epth =	3.22	2" for	2-yea	ar event	
Inflow	=	0.08 cfs @	12.08 hrs,	Volume	=	0.006	af		•		
Outflow	=	0.08 cfs @	12.08 hrs,	Volume	=	0.006	af, /	Atten= C	)%, L	.ag= 0.0	min
Primary	=	0.08 cfs @	12.08 hrs,	Volume	=	0.006	af			0	
Routed	to Pond	4P : SW MG	MT PRACI	ΓICE							
Secondary	=	0.00 cfs @	0.00 hrs,	Volume	=	0.000	af				
Routed	to Reac	h 7R : Reach	-1								

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs Peak Elev= 17.94' @ 12.08 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	17.75'	6.0" Round Culvert to SW Mgmt Facility L= 3.5' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 17.75' / 16.25' S= 0.4286 '/' Cc= 0.900 n= 0.012, Flow Area= 0.20 sf
#2	Secondary	18.00'	6.0" Round Culvert to Flow Spreader L= 27.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 18.00' / 15.25' S= 0.1019 '/' Cc= 0.900 n= 0.012, Flow Area= 0.20 sf

Primary OutFlow Max=0.08 cfs @ 12.08 hrs HW=17.94' (Free Discharge) ←1=Culvert to SW Mgmt Facility (Inlet Controls 0.08 cfs @ 1.17 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=17.75' (Free Discharge) 2=Culvert to Flow Spreader (Controls 0.00 cfs)

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Hydrograph Inflow 0.08 cfs Outflow Primary 0.08 cfs Inflow Area=0.024 ac Secondary 0.08 cfs 0.085 Peak Elev=17.94' 0.08 0.075 0.07 0.065 0.06 0.055 (cfs) 0.05 0.045 Flov 0.04 0.035 0.03 0.025 0.02 0.015 0.01 0.0 0-2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 Ó Time (hours)

## Pond 7P: Diversion Structure 2B

## Pond 7P: Diversion Structure 2B



Type III 24-hr 2-year Rainfall=3.45"

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## Summary for Link 5L: DESIGN LINE

Inflow /	Area	=	0.560 ac,	15.61% Impe	ervious,	Inflow Dep	th = 0.4	40" for 2	2-year event
Inflow	:	=	0.19 cfs @	12.12 hrs,	Volume	= 0	.019 af		
Primar	y :	=	0.19 cfs @	12.12 hrs,	Volume	= 0	.019 af,	Atten= 0°	%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs



## Link 5L: DESIGN LINE

Type III 24-hr 10-year Rainfall=5.12"

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Time span=0.00-48.00 hrs, dt=0.02 hrs, 2401 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: XDA-1 TO DESIGN LINE Runoff Area=24,402 sf 0.00% Impervious Runoff Depth=1.37" Flow Length=260' Tc=6.0 min CN=60 Runoff=0.81 cfs 0.064 af

Subcatchment 2S: FDA-1 TO DESIGN LINE Runoff Area=17,884 sf 2.19% Impervious Runoff Depth=1.44" Flow Length=399' Tc=6.0 min CN=61 Runoff=0.63 cfs 0.049 af

Subcatchment 3S: FDA-2A TO SW	Runoff Area=5,480 sf 43.41% Impervious Runoff Depth=2.72"
	1C=6.0 min CN=77 Runoi1=0.40 cis 0.029 ai
Subcatchment 6S: FDA-2B	Runoff Area=1,037 sf 100.00% Impervious Runoff Depth=4.88"
	Tc=6.0 min CN=98 Runoff=0.12 cfs 0.010 af
Reach 7R: Reach-1	Avg Flow Depth=0.02' Max Vel=0.10 fps Inflow=0.05 cfs 0.006 af
n=(	0.400 L=55.0' S=0.1273 '/' Capacity=0.43 cfs Outflow=0.03 cfs 0.006 af

 Reach 8R: Reach-2
 Avg. Flow Depth=0.03'
 Max Vel=0.07 fps
 Inflow=0.03 cfs
 0.006 af

 n=0.400
 L=64.0'
 S=0.0391 '/'
 Capacity=0.24 cfs
 Outflow=0.03 cfs
 0.006 af

Pond 4P: SW MGMT PRACTICE Peak Elev=17.02' Storage=902 cf Inflow=0.47 cfs 0.038 af Discarded=0.01 cfs 0.032 af Primary=0.03 cfs 0.005 af Outflow=0.04 cfs 0.037 af

Pond 6P: Diversion Structure 2A Peak Elev=17.72' Inflow=0.40 cfs 0.029 af Primary=0.35 cfs 0.028 af Secondary=0.05 cfs 0.001 af Outflow=0.40 cfs 0.029 af

Pond 7P: Diversion Structure 2B Peak Elev=17.99' Inflow=0.12 cfs 0.010 af Primary=0.12 cfs 0.010 af Secondary=0.00 cfs 0.000 af Outflow=0.12 cfs 0.010 af

Link 5L: DESIGN LINE

Inflow=0.63 cfs 0.055 af Primary=0.63 cfs 0.055 af

Total Runoff Area = 1.120 acRunoff Volume = 0.152 afAverage Runoff Depth = 1.62"92.20% Pervious = 1.033 ac7.80% Impervious = 0.087 ac

Type III 24-hr 10-year Rainfall=5.12"

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## Summary for Subcatchment 1S: XDA-1 TO DESIGN LINE

Runoff = 0.81 cfs @ 12.10 hrs, Volume= 0.064 af, Depth= 1.37"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs Type III 24-hr 10-year Rainfall=5.12"

A	rea (sf)	CN E	Description			
	5,028	73 E	Brush, Goo	d, HSG D		
	19,374	56 E	Brush, Fair,	HSG B		
	24,402	60 V	Veighted A	verage		
	24,402	1	00.00% Pe	ervious Are	а	
Tc	Length	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
3.7	31	0.0516	0.14		Sheet Flow, A-B	
					Grass: Dense n= 0.240 P2= 3.45"	
0.2	36	0.2000	3.13		Shallow Concentrated Flow, B-C	
					Short Grass Pasture Kv= 7.0 fps	
0.2	44	0.1954	3.09		Shallow Concentrated Flow, C-D	
					Short Grass Pasture Kv= 7.0 fps	
0.6	70	0.0786	1.96		Shallow Concentrated Flow, D-E	
	00	0.0400	0.00		Short Grass Pasture Kv= 7.0 fps	
0.2	30	0.2166	3.26		Shallow Concentrated Flow, E-F	
0.0	40	0.0007	0.07		Short Grass Pasture KV= 7.0 tps	
0.2	49	0.0667	3.87		Shallow Concentrated Flow, F-G	
0.0					Grassed Waterway KV= 15.0 fps	
0.9					Direct Entry, IC Factor	
6.0	260	Total				

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#### Subcatchment 1S: XDA-1 TO DESIGN LINE

Type III 24-hr 10-year Rainfall=5.12"

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# Summary for Subcatchment 2S: FDA-1 TO DESIGN LINE

Runoff	=	0.63 cfs @	12.10 hrs,	Volume=	C
Routed	d to Lin	k 5L : DESIGN	LINE		

0.049 af, Depth= 1.44"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs Type III 24-hr 10-year Rainfall=5.12"

_	A	rea (sf)	CN [	Description				
		5,028	73 Brush, Good, HSG D					
		5,993 48 Brush, Good, HSG B						
		5,229 61 >75% Grass cover, Good, HSG B						
*		1,242 61 Deck (use lawn for under deck), HSG B						
*		392	98 F	Pool, HSG	В			
17,884 61 Weighted Average								
17,492 97.81% Pervious Area								
	392 2.19% Impervious Area							
	Тс	Length	Slope	Velocity	Capacity	Description		
_	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)			
	3.7	35	0.0657	0.16		Sheet Flow, A-B		
						Grass: Dense n= 0.240 P2= 3.45"		
	1.0	220	0.0659	3.85		Shallow Concentrated Flow, B-C		
						Grassed Waterway Kv= 15.0 fps		
	0.3	65	0.0769	4.16		Shallow Concentrated Flow, C-D		
						Grassed Waterway Kv= 15.0 fps		
	0.2	30	0.2166	3.26		Shallow Concentrated Flow, E-F		
		40	<del>-</del>	o o <del>.</del>		Short Grass Pasture Kv= 7.0 fps		
	0.2	49	0.0667	3.87		Shallow Concentrated Flow, F-G		
	0.0					Grassed Waterway Kv= 15.0 tps		
_	0.6					Direct Entry, IC Factor		
	6.0	399	Total					
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### Subcatchment 2S: FDA-1 TO DESIGN LINE

Type III 24-hr 10-year Rainfall=5.12"

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### Summary for Subcatchment 3S: FDA-2A TO SW PRACTICE

Runoff = 0.40 cfs @ 12.09 hrs, Volume= Routed to Pond 6P : Diversion Structure 2A 0.029 af, Depth= 2.72"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs Type III 24-hr 10-year Rainfall=5.12"

	Area (sf)	CN	Description	Description						
*	2,379	98	Impervious	Impervious surfaces, HSG B						
	3,101	61	>75% Gras	s cover, Go	Good, HSG B					
	5,480	77	Weighted A	Veighted Average						
	3,101		56.59% Pei	56.59% Pervious Area						
	2,379		43.41% lmp	pervious Are	rea					
T (min	c Length	Slop	e Velocity	Capacity	Description					
(1111)		(ועו	l) (ll/sec)	(CIS)						
6.	J				Direct Entry,					

### Subcatchment 3S: FDA-2A TO SW PRACTICE



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### Summary for Subcatchment 6S: FDA-2B

Runoff = 0.12 cfs @ 12.08 hrs, Volume= Routed to Pond 7P : Diversion Structure 2B 0.010 af, Depth= 4.88"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs Type III 24-hr 10-year Rainfall=5.12"



0.02 0.015 0.01 0.005 0 2 4 6 8 Type III 24-hr 10-year Rainfall=5.12"

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### Summary for Reach 7R: Reach-1

Inflow Area = 0.150 ac, 52.42% Impervious, Inflow Depth = 0.46" for 10-year event Inflow 0.05 cfs @ 12.09 hrs, Volume= 0.006 af = 0.03 cfs @ 13.75 hrs, Volume= Outflow 0.006 af, Atten= 43%, Lag= 99.4 min = Routed to Reach 8R : Reach-2 Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs Max. Velocity= 0.10 fps, Min. Travel Time= 9.4 min Avg. Velocity = 0.04 fps, Avg. Travel Time= 25.5 min Peak Storage= 17 cf @ 13.59 hrs Average Depth at Peak Storage= 0.02', Surface Width= 15.12' Bank-Full Depth= 0.10' Flow Area= 1.5 sf, Capacity= 0.43 cfs

15.00' x 0.10' deep channel, n= 0.400 Sheet flow: Woods+light brush Side Slope Z-value= 3.0 '/' Top Width= 15.60' Length= 55.0' Slope= 0.1273 '/' Inlet Invert= 15.00', Outlet Invert= 8.00'

10 12 14 16 18

20

Time (hours)



Capacity=0.43 cfs

22 24 26 28 30 32 34 36 38 40 42 44 46 48

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Reach 7R: Reach-1

Type III 24-hr 10-year Rainfall=5.12"

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### Summary for Reach 8R: Reach-2

[62] Hint: Exceeded Reach 7R OUTLET depth by 0.01' @ 14.20 hrs

 Inflow Area =
 0.150 ac, 52.42% Impervious, Inflow Depth =
 0.46" for 10-year event

 Inflow =
 0.03 cfs @
 13.75 hrs, Volume=
 0.006 af

 Outflow =
 0.03 cfs @
 14.33 hrs, Volume=
 0.006 af, Atten= 11%, Lag= 35.0 min

 Routed to Link 5L : DESIGN LINE
 0.006 af, Atten= 11%, Lag= 35.0 min
 0.006 af, Atten= 11%, Lag= 35.0 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs Max. Velocity= 0.07 fps, Min. Travel Time= 16.4 min Avg. Velocity = 0.02 fps, Avg. Travel Time= 58.7 min

Peak Storage= 26 cf @ 14.06 hrs Average Depth at Peak Storage= 0.03', Surface Width= 15.16' Bank-Full Depth= 0.10' Flow Area= 1.5 sf, Capacity= 0.24 cfs

t

15.00' x 0.10' deep channel, n= 0.400 Sheet flow: Woods+light brush Side Slope Z-value= 3.0 '/' Top Width= 15.60' Length= 64.0' Slope= 0.0391 '/' Inlet Invert= 8.00', Outlet Invert= 5.50'

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Reach 8R: Reach-2

Reach 8R: Reach-2



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Type III 24-hr 10-year Rainfall=5.12"

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# Summary for Pond 4P: SW MGMT PRACTICE

[79] Warning: Submerged Pond 6P Primary device # 1 OUTLET by 1.10' [79] Warning: Submerged Pond 7P Primary device # 1 OUTLET by 0.77'									
Inflow Area =       0.150 ac, 52.42% Impervious, Inflow Depth =       3.02" for 10-year event         Inflow =       0.47 cfs @       12.09 hrs, Volume=       0.038 af         Outflow =       0.04 cfs @       13.31 hrs, Volume=       0.037 af, Atten= 91%, Lag= 73.2 min         Discarded =       0.01 cfs @       11.82 hrs, Volume=       0.032 af         Primary =       0.03 cfs @       13.31 hrs, Volume=       0.005 af         Routed to Reach 7R : Reach-1       0.005 af       0.005 af									
Routing by Peak Elev	Stor-Ind = 17.02' @	method, Time Span 13.31 hrs Surf.A	= 0.00-48.00 hrs, dt= 0.02 hrs rea= 421 sf Storage= 902 cf						
Plug-Flow Center-of-I	detention Vass det.	time= 760.8 min ca time= 747.0 min ( 1	lculated for 0.037 af (98% of inflow) I,556.4 - 809.3)						
Volume	Invert	Avail.Storage	Storage Description						
#1B	13.71'	332 cf	<b>20.83'W x 17.50'L x 3.54'H Field A</b> 1.291 cf Overall - 462 cf Embedded = 829 cf x 40.0% Voids						
#2B	14.21'	462 cf	Cultec R-330XLHD x 8 Inside #1 Effective Size= $47.8$ "W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= $52.0$ "W x 30.5"H x 8.50'L with 1.50' Overlap Devide another Adjustments 14.50' x 7.45 sf x 4 revue						
#3	14.50'	156 cf	6.00'D x 2.75'H Vertical Cone/Cylinder × 2						
		949 cf	Total Available Storage						

Storage Group B created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	17.00'	6.0" Horiz. Orifice/Grate X 2.00 C= 0.600
			Limited to weir flow at low heads
#2	Discarded	13.71'	1.000 in/hr Exfiltration over Horizontal area

**Discarded OutFlow** Max=0.01 cfs @ 11.82 hrs HW=14.51' (Free Discharge) **2=Exfiltration** (Exfiltration Controls 0.01 cfs)

**Primary OutFlow** Max=0.02 cfs @ 13.31 hrs HW=17.02' (Free Discharge) **1=Orifice/Grate** (Weir Controls 0.02 cfs @ 0.43 fps)

Type III 24-hr 10-year Rainfall=5.12"

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## Pond 4P: SW MGMT PRACTICE - Chamber Wizard Field A

### Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 4 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

2 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 15.50' Row Length +12.0" End Stone x 2 = 17.50' Base Length 4 Rows x 52.0" Wide + 6.0" Spacing x 3 + 12.0" Side Stone x 2 = 20.83' Base Width 6.0" Stone Base + 30.5" Chamber Height + 6.0" Stone Cover = 3.54' Field Height

8 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 4 Rows = 462.0 cf Chamber Storage

1,291.2 cf Field - 462.0 cf Chambers = 829.3 cf Stone x 40.0% Voids = 331.7 cf Stone Storage

Chamber Storage + Stone Storage = 793.7 cf = 0.018 afOverall Storage Efficiency = 61.5%Overall System Size =  $17.50' \times 20.83' \times 3.54'$ 

8 Chambers 47.8 cy Field 30.7 cy Stone





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## Pond 4P: SW MGMT PRACTICE

# Pond 4P: SW MGMT PRACTICE



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### Summary for Pond 6P: Diversion Structure 2A

[57] Hint: Peaked at 17.72' (Flood elevation advised)

Inflow Area	a =	0.126 ac, 4	3.41% Impe	ervious, I	nflow Depth =	2.7	2" for	10-year e	event
Inflow	=	0.40 cfs @	12.09 hrs,	Volume=	0.029	) af		•	
Outflow	=	0.40 cfs @	12.09 hrs,	Volume=	0.029	) af,	Atten= 0	%, Lag=	0.0 min
Primary	=	0.35 cfs @	12.09 hrs,	Volume=	0.028	3 af		•	
Routed	to Pond	4P : SW MG	GMT PRACT	ICE					
Secondary	=	0.05 cfs @	12.09 hrs,	Volume=	0.00	l af			
Routed	to Reac	h 7R : Reach	า-1						

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs / 2 Peak Elev= 17.72' @ 12.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	17.33'	<b>8.0" Round Culvert to SW Mgmt Facility</b> L= 3.5' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 17.33' / 15.92' S= 0.4029 '/' Cc= 0.900 n= 0.012, Flow Area= 0.35 sf
#2	Secondary	17.58'	<b>8.0" Round Culvert to Flow Spreader</b> L= 30.5' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 17.58' / 15.08' S= 0.0820 '/' Cc= 0.900 n= 0.012, Flow Area= 0.35 sf

Primary OutFlow Max=0.35 cfs @ 12.09 hrs HW=17.71' (Free Discharge) ←1=Culvert to SW Mgmt Facility (Inlet Controls 0.35 cfs @ 1.67 fps)

Secondary OutFlow Max=0.05 cfs @ 12.09 hrs HW=17.71' (Free Discharge) 2=Culvert to Flow Spreader (Inlet Controls 0.05 cfs @ 0.99 fps)

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Hydrograph Inflow 0.40 cfs Outflow 0.40 cfs Primary Inflow Area=0.126 ac Secondary 0.44 0.42 Peak Elev=17.72' 0.4 0.35 cfs 0.38 0.36 0.34 0.32 0.3 0.28-0.26 (s) 0.26 0.24 0.22 Flow 0.2 0.18 0.16-0.14 0.12 0.1 0.08-0.05 cfs 0.06 0.04 0.02 0-2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 Ó Time (hours)

# Pond 6P: Diversion Structure 2A

# Pond 6P: Diversion Structure 2A



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### Summary for Pond 7P: Diversion Structure 2B

[57] Hint: Peaked at 17.99' (Flood elevation advised)

Inflow Area	a =	0.024 ac,10	0.00% Impe	ervious, I	Inflow Depth	= 4.8	88" for	10-year e	vent
Inflow	=	0.12 cfs @	12.08 hrs,	Volume=	= 0.01	0 af		-	
Outflow	=	0.12 cfs @	12.08 hrs,	Volume=	= 0.01	0 af,	Atten= 0	%, Lag=	0.0 min
Primary	=	0.12 cfs @	12.08 hrs,	Volume=	= 0.01	0 af			
Routed	to Pond	4P : SW MG	MT PRACT	ICE					
Secondary	=	0.00 cfs @	0.00 hrs,	Volume=	= 0.00	0 af			
Routed	to Reac	h 7R : Reach	ı-1						

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs Peak Elev= 17.99' @ 12.08 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	17.75'	6.0" Round Culvert to SW Mgmt Facility L= 3.5' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 17.75' / 16.25' S= 0.4286 '/' Cc= 0.900 n= 0.012, Flow Area= 0.20 sf
#2	Secondary	18.00'	6.0" Round Culvert to Flow Spreader L= 27.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 18.00' / 15.25' S= 0.1019 '/' Cc= 0.900 n= 0.012, Flow Area= 0.20 sf

Primary OutFlow Max=0.12 cfs @ 12.08 hrs HW=17.99' (Free Discharge) ←1=Culvert to SW Mgmt Facility (Inlet Controls 0.12 cfs @ 1.30 fps)

Secondary OutFlow Max=0.00 cfs @ 0.00 hrs HW=17.75' (Free Discharge) 2=Culvert to Flow Spreader (Controls 0.00 cfs)

Hydrograph Inflow 0.12 cfs Outflow Primary Inflow Area=0.024 ac 0.12 cfs 0.12 cfs 0.13 Peak Elev=17.99' 0.12 0.11 0.1 0.09 0.08 (j) 0.08 0.07 Flow 0.06 0.05 0.04 0.03 0.02 0.01 0 2 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 Ó 4 Time (hours)

# Pond 7P: Diversion Structure 2B

# Pond 7P: Diversion Structure 2B



Secondary

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Type III 24-hr 10-year Rainfall=5.12"

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# Summary for Link 5L: DESIGN LINE

Inflow Area	a =	0.560 ac,	15.61% Impe	ervious,	Inflow Depth =	1.1	18" for 10	-year event
Inflow	=	0.63 cfs @	12.10 hrs,	Volume	= 0.055	5 af		
Primary	=	0.63 cfs @	12.10 hrs,	Volume	= 0.05	5 af,	Atten= 0%	, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs



# Link 5L: DESIGN LINE

Type III 24-hr 25-year Rainfall=6.41"

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Time span=0.00-48.00 hrs, dt=0.02 hrs, 2401 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: XDA-1 TO DESIGN LINE Runoff Area=24,402 sf 0.00% Impervious Runoff Depth=2.19" Flow Length=260' Tc=6.0 min CN=60 Runoff=1.38 cfs 0.102 af

Subcatchment 2S: FDA-1 TO DESIGN LINE Runoff Area=17,884 sf 2.19% Impervious Runoff Depth=2.28" Flow Length=399' Tc=6.0 min CN=61 Runoff=1.06 cfs 0.078 af

Subcatchment 3S: FDA-2A TO SW	Runoff Area=5,480 sf 43.41% Impervious Runoff Depth=3.84"
	Tc=6.0 min CN=77 Runoff=0.56 cfs 0.040 af
Subcatchment 6S: FDA-2B	Runoff Area=1,037 sf 100.00% Impervious Runoff Depth=6.17"
	Tc=6.0 min CN=98 Runoff=0.15 cfs 0.012 af
Reach 7R: Reach-1	Avg. Flow Depth=0.07' Max Vel=0.22 fps Inflow=0.28 cfs 0.019 af
n=0.40	00 L=55.0' S=0.1273 '/' Capacity=0.43 cfs Outflow=0.23 cfs 0.019 af

 Reach 8R: Reach-2
 Avg. Flow Depth=0.09'
 Max Vel=0.14 fps
 Inflow=0.23 cfs
 0.019 af

 n=0.400
 L=64.0'
 S=0.0391 '/'
 Capacity=0.24 cfs
 Outflow=0.19 cfs
 0.019 af

Pond 4P: SW MGMT PRACTICE Peak Elev=17.09' Storage=916 cf Inflow=0.60 cfs 0.051 af Discarded=0.01 cfs 0.032 af Primary=0.28 cfs 0.017 af Outflow=0.28 cfs 0.050 af

Pond 6P: Diversion Structure 2A Peak Elev=17.78' Inflow=0.56 cfs 0.040 af Primary=0.46 cfs 0.039 af Secondary=0.11 cfs 0.001 af Outflow=0.56 cfs 0.040 af

Pond 7P: Diversion Structure 2B Peak Elev=18.02' Inflow=0.15 cfs 0.012 af Primary=0.15 cfs 0.012 af Secondary=0.00 cfs 0.000 af Outflow=0.15 cfs 0.012 af

Link 5L: DESIGN LINE

Inflow=1.06 cfs 0.097 af Primary=1.06 cfs 0.097 af

Total Runoff Area = 1.120 acRunoff Volume = 0.233 afAverage Runoff Depth = 2.50"92.20% Pervious = 1.033 ac7.80% Impervious = 0.087 ac

Type III 24-hr 25-year Rainfall=6.41"

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# Summary for Subcatchment 1S: XDA-1 TO DESIGN LINE

Runoff = 1.38 cfs @ 12.10 hrs, Volume= 0.102 af, Depth= 2.19"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs Type III 24-hr 25-year Rainfall=6.41"

A	rea (sf)	CN E	Description			
	5,028	73 E	Brush, Goo	d, HSG D		
	19,374	56 E	Brush, Fair,	HSG B		
	24,402	60 V	Veighted A	verage		
	24,402	1	00.00% Pe	ervious Are	а	
Tc	Length	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
3.7	31	0.0516	0.14		Sheet Flow, A-B	
					Grass: Dense n= 0.240 P2= 3.45"	
0.2	36	0.2000	3.13		Shallow Concentrated Flow, B-C	
					Short Grass Pasture Kv= 7.0 fps	
0.2	44	0.1954	3.09		Shallow Concentrated Flow, C-D	
					Short Grass Pasture Kv= 7.0 fps	
0.6	70	0.0786	1.96		Shallow Concentrated Flow, D-E	
	00	0.0400	0.00		Short Grass Pasture Kv= 7.0 fps	
0.2	30	0.2166	3.26		Shallow Concentrated Flow, E-F	
0.0	40	0.0007	0.07		Short Grass Pasture KV= 7.0 tps	
0.2	49	0.0667	3.87		Shallow Concentrated Flow, F-G	
0.0					Grassed Waterway KV= 15.0 fps	
0.9					Direct Entry, IC Factor	
6.0	260	Total				

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### Subcatchment 1S: XDA-1 TO DESIGN LINE



Type III 24-hr 25-year Rainfall=6.41"

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# Summary for Subcatchment 2S: FDA-1 TO DESIGN LINE

Runoff	=	1.06 cfs @	12.09 hrs,	Volume=
Route	d to Lir	nk 5L : DESIGN	LINE	

0.078 af, Depth= 2.28"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs Type III 24-hr 25-year Rainfall=6.41"

	A	rea (sf)	CN	Description							
		5,028	73	Brush, Goo	d, HSG D						
		5,993	48	Brush, Good, HSG B							
		5,229	61	>75% Grass cover, Good, HSG B							
*		1,242	61	Deck (use l	awn for und	der deck), HSG B					
*		392	98	Pool, HSG	В						
		17,884	61	Weighted A	verage						
		17,492		97.81% Pei	rvious Area						
		392		2.19% Impe	ervious Area	a					
	Tc	Length	Slope	e Velocity	Capacity	Description					
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)						
	3.7	35	0.0657	0.16		Sheet Flow, A-B					
						Grass: Dense n= 0.240 P2= 3.45"					
	1.0	220	0.0659	3.85		Shallow Concentrated Flow, B-C					
						Grassed Waterway Kv= 15.0 fps					
	0.3	65	0.0769	4.16		Shallow Concentrated Flow, C-D					
						Grassed Waterway Kv= 15.0 fps					
	0.2	30	0.2166	3.26		Shallow Concentrated Flow, E-F					
						Short Grass Pasture Kv= 7.0 fps					
	0.2	49	0.0667	3.87		Shallow Concentrated Flow, F-G					
						Grassed waterway Kv= 15.0 tps					
	0.6					Direct Entry, Ic Factor					
	6.0	399	Total								

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### Subcatchment 2S: FDA-1 TO DESIGN LINE

Type III 24-hr 25-year Rainfall=6.41"

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### Summary for Subcatchment 3S: FDA-2A TO SW PRACTICE

Runoff = 0.56 cfs @ 12.09 hrs, Volume= Routed to Pond 6P : Diversion Structure 2A 0.040 af, Depth= 3.84"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs Type III 24-hr 25-year Rainfall=6.41"

	Area (sf)	CN	Description							
*	2,379	98	Impervious	Impervious surfaces, HSG B						
	3,101	61	>75% Gras	75% Grass cover, Good, HSG B						
	5,480	77	Weighted A	verage						
	3,101		56.59% Per	vious Area	а					
	2,379		43.41% Imp	pervious Are	rea					
T (mir	c Length ) (feet)	Slop (ft/f	e Velocity t) (ft/sec)	Capacity (cfs)	/ Description					
6.	0				Direct Entry,					

# Subcatchment 3S: FDA-2A TO SW PRACTICE



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### Summary for Subcatchment 6S: FDA-2B

Runoff = 0.15 cfs @ 12.08 hrs, Volume= Routed to Pond 7P : Diversion Structure 2B 0.012 af, Depth= 6.17"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs Type III 24-hr 25-year Rainfall=6.41"



Type III 24-hr 25-year Rainfall=6.41"

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### Summary for Reach 7R: Reach-1

Inflow Area = 0.150 ac, 52.42% Impervious, Inflow Depth = 1.50" for 25-year event Inflow = 0.28 cfs @ 12.32 hrs, Volume= 0.019 af Outflow = 0.23 cfs @ 12.47 hrs, Volume= 0.019 af, Atten= 19%, Lag= 8.9 min Routed to Reach 8R : Reach-2 Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs Max. Velocity= 0.22 fps, Min. Travel Time= 4.2 min Avg. Velocity = 0.05 fps, Avg. Travel Time= 18.8 min

Peak Storage= 57 cf @ 12.40 hrs Average Depth at Peak Storage= 0.07', Surface Width= 15.41' Bank-Full Depth= 0.10' Flow Area= 1.5 sf, Capacity= 0.43 cfs

15.00' x 0.10' deep channel, n= 0.400 Sheet flow: Woods+light brush Side Slope Z-value= 3.0 '/' Top Width= 15.60' Length= 55.0' Slope= 0.1273 '/' Inlet Invert= 15.00', Outlet Invert= 8.00'





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Reach 7R: Reach-1

Type III 24-hr 25-year Rainfall=6.41"

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### Summary for Reach 8R: Reach-2

[62] Hint: Exceeded Reach 7R OUTLET depth by 0.04' @ 12.64 hrs

 Inflow Area =
 0.150 ac, 52.42% Impervious, Inflow Depth =
 1.50" for 25-year event

 Inflow =
 0.23 cfs @
 12.47 hrs, Volume=
 0.019 af

 Outflow =
 0.19 cfs @
 12.70 hrs, Volume=
 0.019 af, Atten=

 Routed to Link 5L : DESIGN LINE
 DESIGN LINE
 0.019 af, Atten=

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs Max. Velocity= 0.14 fps, Min. Travel Time= 7.5 min Avg. Velocity = 0.02 fps, Avg. Travel Time= 43.1 min

Peak Storage= 84 cf @ 12.57 hrs Average Depth at Peak Storage= 0.09', Surface Width= 15.52' Bank-Full Depth= 0.10' Flow Area= 1.5 sf, Capacity= 0.24 cfs

t

15.00' x 0.10' deep channel, n= 0.400 Sheet flow: Woods+light brush Side Slope Z-value= 3.0 '/' Top Width= 15.60' Length= 64.0' Slope= 0.0391 '/' Inlet Invert= 8.00', Outlet Invert= 5.50'

Type III 24-hr 25-year Rainfall=6.41"

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Reach 8R: Reach-2

Reach 8R: Reach-2



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Type III 24-hr 25-year Rainfall=6.41"

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# Summary for Pond 4P: SW MGMT PRACTICE

[79] Warning: Submerged Pond 6P Primary device # 1 OUTLET by 1.17' [79] Warning: Submerged Pond 7P Primary device # 1 OUTLET by 0.84'								
Inflow Area = 0.150 ac, 52.42% Impervious, Inflow Depth = 4.09" for 25-year event Inflow = 0.60 cfs @ 12.09 hrs, Volume= 0.051 af Outflow = 0.28 cfs @ 12.32 hrs, Volume= 0.050 af, Atten= 53%, Lag= 14.3 min Discarded = 0.01 cfs @ 11.48 hrs, Volume= 0.032 af Primary = 0.28 cfs @ 12.32 hrs, Volume= 0.017 af Routed to Reach 7R : Reach-1								
Routing by Peak Elev=	Stor-Ind = 17.09' @	method, Time Span 0 12.32 hrs Surf.Ai	= 0.00-48.00 hrs, dt= 0.02 hrs rea= 421 sf Storage= 916 cf					
Plug-Flow Center-of-N	detention Mass det.	time= 576.6 min ca time= 559.7 min ( 1	lculated for 0.050 af (97% of inflow) ,363.3 - 803.6)					
Volume	Invert	Avail.Storage	Storage Description					
#1B	13.71'	332 cf	20.83'W x 17.50'L x 3.54'H Field A					
#2B	14.21'	462 cf	Cultec R-330XLHD x 8 Inside #1 Effective Size= $47.8$ "W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= $52.0$ "W x 30.5"H x 8.50'L with 1.50' Overlap					
#3	14.50'	156 cf	6.00'D x 2.75'H Vertical Cone/Cylinder x 2					
		949 cf	Total Available Storage					

Storage Group B created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	17.00'	6.0" Horiz. Orifice/Grate X 2.00 C= 0.600
			Limited to weir flow at low heads
#2	Discarded	13.71'	1.000 in/hr Exfiltration over Horizontal area

**Discarded OutFlow** Max=0.01 cfs @ 11.48 hrs HW=14.50' (Free Discharge) **2=Exfiltration** (Exfiltration Controls 0.01 cfs)

Primary OutFlow Max=0.27 cfs @ 12.32 hrs HW=17.09' (Free Discharge) 1=Orifice/Grate (Weir Controls 0.27 cfs @ 0.97 fps)

Type III 24-hr 25-year Rainfall=6.41"

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## Pond 4P: SW MGMT PRACTICE - Chamber Wizard Field A

### Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 4 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

2 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 15.50' Row Length +12.0" End Stone x 2 = 17.50' Base Length
4 Rows x 52.0" Wide + 6.0" Spacing x 3 + 12.0" Side Stone x 2 = 20.83' Base Width
6.0" Stone Base + 30.5" Chamber Height + 6.0" Stone Cover = 3.54' Field Height

8 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 4 Rows = 462.0 cf Chamber Storage

1,291.2 cf Field - 462.0 cf Chambers = 829.3 cf Stone x 40.0% Voids = 331.7 cf Stone Storage

Chamber Storage + Stone Storage = 793.7 cf = 0.018 afOverall Storage Efficiency = 61.5%Overall System Size =  $17.50' \times 20.83' \times 3.54'$ 

8 Chambers 47.8 cy Field 30.7 cy Stone





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### Pond 4P: SW MGMT PRACTICE





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### Summary for Pond 6P: Diversion Structure 2A

[57] Hint: Peaked at 17.78' (Flood elevation advised)

Inflow Area	a =	0.126 ac, 4	3.41% Impe	ervious, Infl	ow Depth =	3.84"	for 25-	year event
Inflow	=	0.56 cfs @	12.09 hrs,	Volume=	0.040	af		-
Outflow	=	0.56 cfs @	12.09 hrs,	Volume=	0.040	af, Att	ten= 0%,	Lag= 0.0 min
Primary	=	0.46 cfs @	12.09 hrs,	Volume=	0.039	af		-
Routed	to Pond	4P : SW MG	<b>MT PRACT</b>	ICE				
Secondary	=	0.11 cfs @	12.09 hrs,	Volume=	0.001	af		
Routed	to Reac	h 7R : Reach	n-1					

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs / 2 Peak Elev= 17.78' @ 12.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	17.33'	<b>8.0" Round Culvert to SW Mgmt Facility</b> L= 3.5' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 17.33' / 15.92' S= 0.4029 '/' Cc= 0.900 n= 0.012, Flow Area= 0.35 sf
#2	Secondary	17.58'	8.0" Round Culvert to Flow Spreader L= 30.5' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 17.58' / 15.08' S= 0.0820 '/' Cc= 0.900 n= 0.012, Flow Area= 0.35 sf

Primary OutFlow Max=0.45 cfs @ 12.09 hrs HW=17.78' (Free Discharge) ←1=Culvert to SW Mgmt Facility (Inlet Controls 0.45 cfs @ 1.81 fps)

Secondary OutFlow Max=0.11 cfs @ 12.09 hrs HW=17.78' (Free Discharge) 2=Culvert to Flow Spreader (Inlet Controls 0.11 cfs @ 1.21 fps)

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# Pond 6P: Diversion Structure 2A





Type III 24-hr 25-year Rainfall=6.41"

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## Summary for Pond 7P: Diversion Structure 2B

[57] Hint: Peaked at 18.02' (Flood elevation advised)

Inflow Area	a =	0.024 ac,10	0.00% Imp	ervious,	Inflow De	epth =	6.1	7" for 25	5-year eve	nt
Inflow	=	0.15 cfs @	12.08 hrs,	Volume=	=	0.012	af		•	
Outflow	=	0.15 cfs @	12.08 hrs,	Volume=	=	0.012	af, .	Atten= 0%	, Lag= 0.0	) min
Primary	=	0.15 cfs @	12.08 hrs,	Volume=	=	0.012	af		· ·	
Routed	to Pond	4P : SW MG	MT PRACI	ΓICE						
Secondary	=	0.00 cfs @	12.08 hrs,	Volume=	=	0.000	af			
Routed	to Reac	h 7R : Reach	-1							

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs Peak Elev= 18.02' @ 12.08 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	17.75'	6.0" Round Culvert to SW Mgmt Facility L= 3.5' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 17.75' / 16.25' S= 0.4286 '/' Cc= 0.900 n= 0.012, Flow Area= 0.20 sf
#2	Secondary	18.00'	6.0" Round Culvert to Flow Spreader L= 27.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 18.00' / 15.25' S= 0.1019 '/' Cc= 0.900 n= 0.012, Flow Area= 0.20 sf

Primary OutFlow Max=0.15 cfs @ 12.08 hrs HW=18.02' (Free Discharge) ←1=Culvert to SW Mgmt Facility (Inlet Controls 0.15 cfs @ 1.39 fps)

Secondary OutFlow Max=0.00 cfs @ 12.08 hrs HW=18.02' (Free Discharge) 2=Culvert to Flow Spreader (Inlet Controls 0.00 cfs @ 0.35 fps)

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## Pond 7P: Diversion Structure 2B





Type III 24-hr 25-year Rainfall=6.41"

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# Summary for Link 5L: DESIGN LINE

Inflow /	Area =	0.560 ac, 1	15.61% Impervious,	Inflow Depth = 2.0	07" for 25-year event
Inflow	=	1.06 cfs @	12.09 hrs, Volume	= 0.097 af	
Primary	y =	1.06 cfs @	12.09 hrs, Volume	= 0.097 af,	Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs



# Link 5L: DESIGN LINE

Type III 24-hr 100-year Rainfall=9.03"

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Time span=0.00-48.00 hrs, dt=0.02 hrs, 2401 points Runoff by SCS TR-20 method, UH=SCS, Weighted-CN Reach routing by Stor-Ind+Trans method - Pond routing by Stor-Ind method

Subcatchment 1S: XDA-1 TO DESIGN LINE Runoff Area=24,402 sf 0.00% Impervious Runoff Depth=4.12" Flow Length=260' Tc=6.0 min CN=60 Runoff=2.68 cfs 0.193 af

Subcatchment 2S: FDA-1 TO DESIGN LINE Runoff Area=17,884 sf 2.19% Impervious Runoff Depth=4.25" Flow Length=399' Tc=6.0 min CN=61 Runoff=2.03 cfs 0.145 af

Subcatchment 3S: FDA-2A TO	SW Runoff Area=5,480 sf 43.41% Impervious Runoff Depth=6.23"
	Tc=6.0 min CN=77 Runoff=0.91 cfs 0.065 af
Subcatchment 6S: FDA-2B	Runoff Area=1,037 sf 100.00% Impervious Runoff Depth=8.79"
	Tc=6.0 min CN=98 Runoff=0.21 cfs 0.017 af
Reach 7R: Reach-1	Avg. Flow Depth=0.18' Max Vel=0.36 fps Inflow=1.09 cfs 0.047 af
	n=0.400 L=55.0' S=0.1273 '/' Capacity=0.43 cfs Outflow=0.98 cfs 0.047 af

 Reach 8R: Reach-2
 Avg. Flow Depth=0.23'
 Max Vel=0.21 fps
 Inflow=0.98 cfs
 0.047 af

 n=0.400
 L=64.0'
 S=0.0391 '/'
 Capacity=0.24 cfs
 Outflow=0.75 cfs
 0.047 af

Pond 4P: SW MGMT PRACTICE Peak Elev=17.19' Storage=938 cf Inflow=0.85 cfs 0.078 af Discarded=0.01 cfs 0.034 af Primary=0.82 cfs 0.043 af Outflow=0.83 cfs 0.076 af

Pond 6P: Diversion Structure 2A Peak Elev=17.90' Inflow=0.91 cfs 0.065 af Primary=0.65 cfs 0.061 af Secondary=0.26 cfs 0.005 af Outflow=0.91 cfs 0.065 af

Pond 7P: Diversion Structure 2B Peak Elev=18.07' Inflow=0.21 cfs 0.017 af Primary=0.20 cfs 0.017 af Secondary=0.01 cfs 0.000 af Outflow=0.21 cfs 0.017 af

Link 5L: DESIGN LINE

Inflow=2.03 cfs 0.193 af Primary=2.03 cfs 0.193 af

Total Runoff Area = 1.120 acRunoff Volume = 0.421 afAverage Runoff Depth = 4.50"92.20% Pervious = 1.033 ac7.80% Impervious = 0.087 ac

Type III 24-hr 100-year Rainfall=9.03"

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# Summary for Subcatchment 1S: XDA-1 TO DESIGN LINE

Runoff = 2.68 cfs @ 12.09 hrs, Volume= 0.193 af, Depth= 4.12"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs Type III 24-hr 100-year Rainfall=9.03"

A	rea (sf)	CN E	Description			
	5,028	73 E	Brush, Goo	d, HSG D		
	19,374	56 E	Brush, Fair,	HSG B		
	24,402	60 V	Veighted A	verage		
	24,402	1	00.00% Pe	ervious Are	a	
Tc	Length	Slope	Velocity	Capacity	Description	
(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)		
3.7	31	0.0516	0.14		Sheet Flow, A-B	
					Grass: Dense n= 0.240 P2= 3.45"	
0.2	36	0.2000	3.13		Shallow Concentrated Flow, B-C	
					Short Grass Pasture Kv= 7.0 fps	
0.2	44	0.1954	3.09		Shallow Concentrated Flow, C-D	
					Short Grass Pasture Kv= 7.0 fps	
0.6	70	0.0786	1.96		Shallow Concentrated Flow, D-E	
	~~~				Short Grass Pasture Kv= 7.0 fps	
0.2	30	0.2166	3.26		Shallow Concentrated Flow, E-F	
0.0	10	0 0007	0.07		Short Grass Pasture Kv= 7.0 fps	
0.2	49	0.0667	3.87		Shallow Concentrated Flow, F-G	
0.0					Grassed Waterway KV= 15.0 fps	
0.9					Direct Entry, IC Factor	_
6.0	260	Total				
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Type III 24-hr 100-year Rainfall=9.03"

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## Summary for Subcatchment 2S: FDA-1 TO DESIGN LINE

Runoff = 2.03 cfs @ 12.09 hrs, Volume= Routed to Link 5L : DESIGN LINE

0.145 af, Depth= 4.25"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs Type III 24-hr 100-year Rainfall=9.03"

	A	rea (sf)	CN I	Description					
		5,028	73	Brush, Goo	d, HSG D				
		5,993	48	Brush, Goo	d, HSG B				
	5,229 61 >75% Grass cover, Good, HSG B								
*		1,242	61	Deck (use l	awn for und	der deck), HSG B			
*		392	98	Pool, HSG	В				
		17,884	61	Weighted A	verage				
		17,492	9	97.81% Pe	rvious Area				
		392	1	2.19% Impe	ervious Area	а			
	Tc	Length	Slope	Velocity	Capacity	Description			
	(min)	(feet)	(ft/ft)	(ft/sec)	(cfs)				
	3.7	35	0.0657	0.16		Sheet Flow, A-B			
						Grass: Dense n= 0.240 P2= 3.45"			
	1.0	220	0.0659	3.85		Shallow Concentrated Flow, B-C			
						Grassed Waterway Kv= 15.0 fps			
	0.3	65	0.0769	4.16		Shallow Concentrated Flow, C-D			
						Grassed Waterway Kv= 15.0 fps			
	0.2	30	0.2166	3.26		Shallow Concentrated Flow, E-F			
						Short Grass Pasture Kv= 7.0 fps			
	0.2	49	0.0667	3.87		Shallow Concentrated Flow, F-G			
						Grassed Waterway Kv= 15.0 fps			
_	0.6					Direct Entry, Tc Factor			
	6.0	399	Total						

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Type III 24-hr 100-year Rainfall=9.03"

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#### Summary for Subcatchment 3S: FDA-2A TO SW PRACTICE

Runoff = 0.91 cfs @ 12.09 hrs, Volume= Routed to Pond 6P : Diversion Structure 2A 0.065 af, Depth= 6.23"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs Type III 24-hr 100-year Rainfall=9.03"

	Area (sf)	CN	Description							
*	2,379	98	Impervious	surfaces, H	HSG B					
	3,101	61	>75% Gras	75% Grass cover, Good, HSG B						
	5,480	77	Weighted A	verage						
	3,101		56.59% Per	vious Area	а					
	2,379		43.41% Imp	rea						
- (mi	Tc Length n) (feet)	Slop (ft/f	e Velocity t) (ft/sec)	Capacity (cfs)	Description					
6	0.0				Direct Entry,					

### Subcatchment 3S: FDA-2A TO SW PRACTICE



Type III 24-hr 100-year Rainfall=9.03"

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#### Summary for Subcatchment 6S: FDA-2B

Runoff = 0.21 cfs @ 12.08 hrs, Volume= Routed to Pond 7P : Diversion Structure 2B 0.017 af, Depth= 8.79"

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs Type III 24-hr 100-year Rainfall=9.03"



Type III 24-hr 100-year Rainfall=9.03"

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### Summary for Reach 7R: Reach-1

[91] Warning: Storage range exceeded by 0.08'

[55] Hint: Peak inflow is 252% of Manning's capacity[79] Warning: Submerged Pond 6P Secondary device # 2 OUTLET by 0.10'

 Inflow Area =
 0.150 ac, 52.42% Impervious, Inflow Depth =
 3.81" for 100-year event

 Inflow =
 1.09 cfs @
 12.09 hrs, Volume=
 0.047 af

 Outflow =
 0.98 cfs @
 12.18 hrs, Volume=
 0.047 af, Atten=
 10%, Lag=

 Routed to Reach 8R : Reach-2
 8
 10%
 10%
 10%
 10%

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs Max. Velocity= 0.36 fps, Min. Travel Time= 2.5 min Avg. Velocity = 0.06 fps, Avg. Travel Time= 14.9 min

Peak Storage= 150 cf @ 12.14 hrs Average Depth at Peak Storage= 0.18', Surface Width= 16.06' Bank-Full Depth= 0.10' Flow Area= 1.5 sf, Capacity= 0.43 cfs

**t** 

15.00' x 0.10' deep channel, n= 0.400 Sheet flow: Woods+light brush Side Slope Z-value= 3.0 '/' Top Width= 15.60' Length= 55.0' Slope= 0.1273 '/' Inlet Invert= 15.00', Outlet Invert= 8.00'

Type III 24-hr 100-year Rainfall=9.03"

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Reach 7R: Reach-1

Reach 7R: Reach-1



Type III 24-hr 100-year Rainfall=9.03"

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#### Summary for Reach 8R: Reach-2

[91] Warning: Storage range exceeded by 0.13'
[55] Hint: Peak inflow is 410% of Manning's capacity
[62] Hint: Exceeded Reach 7R OUTLET depth by 0.11' @ 12.28 hrs

 Inflow Area =
 0.150 ac, 52.42% Impervious, Inflow Depth =
 3.81" for 100-year event

 Inflow =
 0.98 cfs @
 12.18 hrs, Volume=
 0.047 af

 Outflow =
 0.75 cfs @
 12.34 hrs, Volume=
 0.047 af, Atten= 23%, Lag= 9.5 min

 Routed to Link 5L : DESIGN LINE
 0.047 af, Atten= 23%, Lag= 9.5 min
 0.047 af, Atten= 23%, Lag= 9.5 min

Routing by Stor-Ind+Trans method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs Max. Velocity= 0.21 fps, Min. Travel Time= 5.0 min Avg. Velocity = 0.03 fps, Avg. Travel Time= 33.7 min

Peak Storage= 228 cf @ 12.25 hrs Average Depth at Peak Storage= 0.23', Surface Width= 16.38' Bank-Full Depth= 0.10' Flow Area= 1.5 sf, Capacity= 0.24 cfs

t

15.00' x 0.10' deep channel, n= 0.400 Sheet flow: Woods+light brush Side Slope Z-value= 3.0 '/' Top Width= 15.60' Length= 64.0' Slope= 0.0391 '/' Inlet Invert= 8.00', Outlet Invert= 5.50'

Type III 24-hr 100-year Rainfall=9.03"

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Reach 8R: Reach-2

Reach 8R: Reach-2



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Type III 24-hr 100-year Rainfall=9.03"

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### Summary for Pond 4P: SW MGMT PRACTICE

[79] Warning: Submerged Pond 6P Primary device # 1 OUTLET by 1.27' [79] Warning: Submerged Pond 7P Primary device # 1 OUTLET by 0.94'									
Inflow Area =       0.150 ac, 52.42% Impervious, Inflow Depth =       6.27" for 100-year event         Inflow =       0.85 cfs @       12.09 hrs, Volume=       0.078 af         Outflow =       0.83 cfs @       12.10 hrs, Volume=       0.076 af, Atten= 2%, Lag= 1.0 min         Discarded =       0.01 cfs @       10.42 hrs, Volume=       0.034 af         Primary =       0.82 cfs @       12.10 hrs, Volume=       0.043 af         Routed to Reach 7R : Reach-1       0.043 af       0.043 af									
Routing by Peak Elev=	Stor-Ind = 17.19' @	method, Time Span 12.10 hrs Surf.Ai	= 0.00-48.00 hrs, dt= 0.02 hrs rea= 421 sf Storage= 938 cf						
Plug-Flow of Center-of-N	detention Mass det.	time= 383.6 min ca time= 370.6 min ( 1	lculated for 0.076 af (98% of inflow) ,165.7 - 795.1)						
Volume	Invert	Avail.Storage	Storage Description						
#1B	13.71'	332 cf	<b>20.83'W x 17.50'L x 3.54'H Field A</b> 1.291 cf Overall - 462 cf Embedded = 829 cf x 40.0% Voids						
#2B	14.21'	462 cf	Cultec R-330XLHD x 8 Inside #1 Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 4 rows						
#3	14.50'	156 cf	6.00'D x 2.75'H Vertical Cone/Cylinder × 2						
		949 cf	Total Available Storage						

Storage Group B created with Chamber Wizard

Device	Routing	Invert	Outlet Devices
#1	Primary	17.00'	6.0" Horiz. Orifice/Grate X 2.00 C= 0.600
			Limited to weir flow at low heads
#2	Discarded	13.71'	1.000 in/hr Exfiltration over Horizontal area

**Discarded OutFlow** Max=0.01 cfs @ 10.42 hrs HW=14.50' (Free Discharge) **2=Exfiltration** (Exfiltration Controls 0.01 cfs)

**Primary OutFlow** Max=0.83 cfs @ 12.10 hrs HW=17.19' (Free Discharge) **1=Orifice/Grate** (Orifice Controls 0.83 cfs @ 2.12 fps)

Type III 24-hr 100-year Rainfall=9.03"

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### Pond 4P: SW MGMT PRACTICE - Chamber Wizard Field A

#### Chamber Model = Cultec R-330XLHD (Cultec Recharger® 330XLHD)

Effective Size= 47.8"W x 30.0"H => 7.45 sf x 7.00'L = 52.2 cf Overall Size= 52.0"W x 30.5"H x 8.50'L with 1.50' Overlap Row Length Adjustment= +1.50' x 7.45 sf x 4 rows

52.0" Wide + 6.0" Spacing = 58.0" C-C Row Spacing

2 Chambers/Row x 7.00' Long +1.50' Row Adjustment = 15.50' Row Length +12.0" End Stone x 2 = 17.50' Base Length 4 Rows x 52.0" Wide + 6.0" Spacing x 3 + 12.0" Side Stone x 2 = 20.83' Base Width 6.0" Stone Base + 30.5" Chamber Height + 6.0" Stone Cover = 3.54' Field Height

8 Chambers x 52.2 cf +1.50' Row Adjustment x 7.45 sf x 4 Rows = 462.0 cf Chamber Storage

1,291.2 cf Field - 462.0 cf Chambers = 829.3 cf Stone x 40.0% Voids = 331.7 cf Stone Storage

Chamber Storage + Stone Storage = 793.7 cf = 0.018 afOverall Storage Efficiency = 61.5%Overall System Size =  $17.50' \times 20.83' \times 3.54'$ 

8 Chambers 47.8 cy Field 30.7 cy Stone





Type III 24-hr 100-year Rainfall=9.03"

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### Pond 4P: SW MGMT PRACTICE





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#### Summary for Pond 6P: Diversion Structure 2A

[57] Hint: Peaked at 17.90' (Flood elevation advised)

Inflow Area	a =	0.126 ac, 4	3.41% Impe	ervious,	Inflow Dep	th =	6.2	3" for	100	-year e	event
Inflow	=	0.91 cfs @	12.09 hrs,	Volume	= 0	.065	af			-	
Outflow	=	0.91 cfs @	12.09 hrs,	Volume	= 0	.065	af, .	Atten= 0	)%,	Lag= (	0.0 min
Primary	=	0.65 cfs @	12.09 hrs,	Volume	= 0	.061	af			•	
Routed	to Pond	4P : SW MG	<b>MT PRACT</b>	ICE							
Secondary	=	0.26 cfs @	12.09 hrs,	Volume	= 0	.005	af				
Routed	to Reac	h 7R : Reach	n-1								

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs / 2 Peak Elev= 17.90' @ 12.09 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	17.33'	<b>8.0" Round Culvert to SW Mgmt Facility</b> L= 3.5' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 17.33' / 15.92' S= 0.4029 '/' Cc= 0.900 n= 0.012, Flow Area= 0.35 sf
#2	Secondary	17.58'	8.0" Round Culvert to Flow Spreader L= 30.5' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 17.58' / 15.08' S= 0.0820 '/' Cc= 0.900 n= 0.012, Flow Area= 0.35 sf

Primary OutFlow Max=0.65 cfs @ 12.09 hrs HW=17.90' (Free Discharge) ←1=Culvert to SW Mgmt Facility (Inlet Controls 0.65 cfs @ 2.03 fps)

Secondary OutFlow Max=0.25 cfs @ 12.09 hrs HW=17.90' (Free Discharge) 2=Culvert to Flow Spreader (Inlet Controls 0.25 cfs @ 1.52 fps)

Type III 24-hr 100-year Rainfall=9.03"

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Hydrograph Inflow Outflow 0.91 cfs Primary Inflow Area=0.126 ac 0.91 cfs Secondary 1 Peak Elev=17.90' 0.65 (cfs) Flov 0.26 c 0-4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 ż Ó Time (hours)

# Pond 6P: Diversion Structure 2A

# Pond 6P: Diversion Structure 2A



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#### Summary for Pond 7P: Diversion Structure 2B

[57] Hint: Peaked at 18.07' (Flood elevation advised)

Inflow Area	a =	0.024 ac,10	0.00% Impe	ervious,	Inflow De	pth =	8.7	9" for	100-	-year e	vent
Inflow	=	0.21 cfs @	12.08 hrs,	Volume=	=	0.017	af			•	
Outflow	=	0.21 cfs @	12.08 hrs,	Volume=	=	0.017	af, J	Atten= 0	%, I	Lag= 0	.0 min
Primary	=	0.20 cfs @	12.08 hrs,	Volume=	=	0.017	af			•	
Routed	to Pond	4P : SW MG	MT PRACT	ICE							
Secondary	=	0.01 cfs @	12.08 hrs,	Volume=	=	0.000	af				
Routed	to Reac	h 7R : Reach	-1								

Routing by Stor-Ind method, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs Peak Elev= 18.07' @ 12.08 hrs

Device	Routing	Invert	Outlet Devices
#1	Primary	17.75'	6.0" Round Culvert to SW Mgmt Facility L= 3.5' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 17.75' / 16.25' S= 0.4286 '/' Cc= 0.900 n= 0.012, Flow Area= 0.20 sf
#2	Secondary	18.00'	6.0" Round Culvert to Flow Spreader L= 27.0' CPP, projecting, no headwall, Ke= 0.900 Inlet / Outlet Invert= 18.00' / 15.25' S= 0.1019 '/' Cc= 0.900 n= 0.012, Flow Area= 0.20 sf

Primary OutFlow Max=0.20 cfs @ 12.08 hrs HW=18.07' (Free Discharge) ←1=Culvert to SW Mgmt Facility (Inlet Controls 0.20 cfs @ 1.51 fps)

Secondary OutFlow Max=0.01 cfs @ 12.08 hrs HW=18.07' (Free Discharge) 2=Culvert to Flow Spreader (Inlet Controls 0.01 cfs @ 0.70 fps)

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Hydrograph Inflow Outflow 0.21 cfs Primary Inflow Area=0.024 ac 0.21 cfs Secondary 0.22 Peak Elev=18.07' 0.20 cfs 0.2 0.18 0.16 0.14 (cfs) 0.12 Flov 0.1 0.08 0.06 0.04 0.02 0.01 cfs 0-2 4 6 8 10 12 14 16 18 20 22 24 26 28 30 32 34 36 38 40 42 44 46 48 Ó Time (hours)

### Pond 7P: Diversion Structure 2B

# Pond 7P: Diversion Structure 2B



Type III 24-hr 100-year Rainfall=9.03"

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# Summary for Link 5L: DESIGN LINE

Inflow A	Area =	0.560 ac,	15.61% Impervious,	Inflow Depth = 4.	13" for 100-year event
Inflow	=	2.03 cfs @	12.09 hrs, Volume	;= 0.193 af	
Primary	y =	2.03 cfs @	12.09 hrs, Volume	⊭ 0.193 af,	Atten= 0%, Lag= 0.0 min

Primary outflow = Inflow, Time Span= 0.00-48.00 hrs, dt= 0.02 hrs



#### Link 5L: DESIGN LINE